Report to Council

To: Mayor & Members of Council
From: Adele Arbour
Date: April 7, 2026
Re: **Interim Report Regarding Response to Public Meeting Comments September 16, 2025**
168 County Road 49 – Official Plan and Zoning By-Law Amendments
Subdivision File 15T-23001 – Jeffery Homes Limited (2549099 Ontario Inc.)

Recommendation:

That Council receive the Interim Report from the Planner regarding Responses to Public Meeting Comments September 16, 2025; and further

That Council direct staff to schedule a second Public Meeting on May 5, 2026, to present the responses to public meeting comments received; and further

That the Public Meeting Notice be circulated to the broader circulation distance to Ellwood Crescent, as required by the Council Resolution of September 16, 2025.

Financial Implications:

There are no financial implications as a result of this report.

Environmental Implications:

There are no environmental implications as a result of this report.

Background:

The lands are the subject of an application for a proposed Plan of Subdivision (58 residential lots) to the County of Peterborough (File 15T-23001) and is also subject to the following planning approvals in relation to the proposed plan of subdivision:

- a County of Peterborough Official Plan Amendment to include the northern portion of the proposed plan of subdivision within a Settlement Area,
- a Trent Lakes Official Plan Amendment to re-designate the north portion of the site to the Hamlet designation and a corresponding Zoning By-Law Amendment to zone the lands to the Hamlet Residential (HR) and Environmental Protection (EP) Zones.

Planscape Inc., on behalf of Jeffrey Homes Limited, has submitted the required applications. The proposed Plan of Subdivision is attached as Appendix A to this report.

Peterborough County is the approval authority for plans of subdivision and condominiums. The applicant has submitted the following technical reports and studies in support of the applications:

- Archaeological Assessment
- Environmental Impact Study
- Hydrogeological Assessment Report
- Geotechnical Study
- Planning Justification Report
- Functional Servicing and Stormwater Management Report
- Servicing Plans and Pre and Post Development Drainage and Erosion and Sediment Control Plans
- Transportation Impact Study

The lands have been the subject of extensive technical assessments to address impacts related to development and to confirm the lands are suitable to accommodate development. These studies have been the subject of peer reviews by both independent professionals and the KRCA. All Peer Reviews were initiated by the County of Peterborough.

A Public Meeting was held on September 16, 2025, to consider the Official Plan and Zoning By-Law Amendment for a proposed Plan of Subdivision consisting of 58 residential lots. The following Council resolution was made:

Resolution No. R2025-348

Moved by Councillor Braybrook
Seconded by Councillor Franzen

That Council receive the report from the Planning Technician for Zoning By-law Amendment File No. 25-14 and Official Plan Amendment File No. 62; and further

That Council defer Zoning By-law Amendment File No. 25-14 and Official Plan Amendment File No. 62 pending resolution of the Minimum Distance Separation (MDS) assessment of the barn located on lands north of the subject lands at 214 County Road 49; and further

That Council direct staff to address issues raised at the public meeting through a staff report at a future meeting; and further

That Council direct staff to increase the circulation area of these files to include all residents of Ellwood Crescent.

Carried.

The Planning Department received a submission from Planscape Inc. on December 3, 2025 in response to the public comments received at the Public Meeting September 16, 2025. The

Planning Department provided a response in reply to this submission, in a letter dated January 14, 2026, which is attached as Appendix B to this report.

Further to the Public Meeting Minutes and notes taken by staff, the following is a summary of comments and concerns that were raised:

- The barn located at 214 County Road 49;
- The potential impact of the development on speed limits on County Road 49;
- Spreading of sewage on neighbouring lands;
- Whether the hydrogeological study pertain only to the subject property or does it take into consideration the surrounding area;
- Stormwater drainage ditches and culverts may not be sufficient;
- Why the entrance off of Moon Line Road intersects a wetland/watercourse;
- Accessory dwelling units could double the residential occupancy;
- Stormwater currently goes south to adjacent property;
- Will there be fencing around the subdivision;
- Why 1,000 litres per day is used as the basis for nitrate attenuation;
- Whether hydrofracking would be required;
- There is a plan of subdivision proposed nearby in Kawartha Lakes is it on private services or municipal services; and,
- Is there another development in the County which has 100 homes with 100 wells.

The above comments and concerns are addressed in the submission attached at Appendix B that has been prepared by the applicant's planning consultant, however the Planning Department wishes to provide the following comments with respect to the barn on the adjacent property and the groundwater/well comments.

The Planning Department addressed the neighbouring vacant barn at the previous public meeting and it was their conclusion based on a site visit that the barn is unlikely to be utilized for livestock given its age and condition. Even if the barn were utilized for livestock, Planscape Inc. has prepared an MDS calculation which would require a limited encroachment into the rear yards of the proposed building lots abutting the northern property line. On this basis, the Municipality could establish an increased rear yard on the affected lots if necessary.

As mentioned above, on January 18, 2026 Planscape Inc. submitted a response which required some additional clarification and information requested by the Planning Department and was discussed with Planscape on January 27, 2026. A revised submission was received by the Municipality February 4, 2026 and is attached as Appendix C to this report.

As part of the response memo, the proponent drilled another test well and also conducted additional test pumping on existing wells. This supplemental field work does not contradict the conclusions of the original hydrogeological reports which have previously been peer reviewed and found to be acceptable on this basis, an additional peer review does not appear to be warranted.

Staff understand that well impact is a concern of neighbouring residents, and this holds true in virtually all subdivision applications. However, the applicant has submitted the requisite technical studies, which have been peer reviewed and found to be acceptable, and the applicant has completed additional assessment to re-confirm prior conclusions of the original studies. Notwithstanding it is noted that one of the major concerns voiced during the public meeting was the lack of water supply in neighbouring private wells. Further testing results provided by the consultant retained by the applicant have indicated that test well TW2 does not have the capacity to maintain the required 13.6 l/min flow for a residential dwelling, this raises the concern for the need for further clarification to storage, as to what is proposed to offset this shortcoming.

The Building Department has also undertaken a review of the proposed development through the lens of the Ontario Building Code. The plan of subdivision has considered and implemented MECP D-5-4 guidelines- (Individual On-Site Sewage Systems: Water Quality Impact Risk Assessment) to assess the potential nitrate attenuation and to keep the concentration limit at or below 10 mg/l for each proposed lot. The proposal indicates 1000 l/day daily design flow for a 4-bedroom dwelling and 490 l/day daily design flow for a proposed ARU, for a total daily design flow of 1490 l/day. It should be noted that the Authority Having Jurisdiction is the Municipality of Trent Lakes and not the Ministry of the Environment, Conservation and Parks when it comes to onsite sewage approvals for this proposed development. All proposed sewage system applications will be evaluated and approved by the Municipality of Trent Lakes using Part 8 of the Ontario Building Code, which clearly notes a 4 bedroom dwelling as 2000 l/d design flow and a 1 bedroom dwelling (assumed for an ARU) as 750 l/day design flow, for a total daily design flow of 2750 l/day minimum. Further clarification was sought from the applicant's agent and technical team as to why there is such a difference with the design criteria.

A meeting was held on February 18, 2026 to discuss municipal staff and Peer Reviewer concerns. Cambium addressed the nitrate assessment for the east side of the development and TW3 pumping test with supplemental reporting submitted to the Municipality on March 11, 2026 and attached as Appendix D. The results show that the east side of the site can support 22 lots with ARUs and can support all 26 lots without ARUs. That leaves 4 lots on the east side of the site that would not be able to have ARUs to meet the nitrate concentration limit of 10 mg/L at the property boundary. There are no other outstanding hydrogeological issues.

There are a number of other privately serviced subdivisions across the County ranging from 16 to 58 lots however, these developments are difficult to compare with this proposal as development is evaluated on its own merits, and on the hydrogeological conditions and characteristics of the site. On this basis, it is difficult to compare this development with other approved developments.

The Planning Department has interpreted the September 16, 2025 Council Resolution, that a further public meeting was envisioned by Council in order to provide the public with answers to their concerns and further that the circulation of the Notice of the Public Meeting be broadened to include property owners on Ellwood Crescent. The Planning Department agrees with this approach as it allows the meeting to a dedicated item on the Council agenda, where members of the public may address the Chair as well as the Consultant and their Technical Team.

Consultations:
Chris Jones, Planning Consultant
County of Peterborough Planning Staff

Appendices:

Appendix A - Proposed Draft Plan of Subdivision
Appendix B - Letter from Planning Staff dated January 14, 2026, Response to Planscape Inc. Submission
Appendix C - Planscape Inc. Final Submission February 4, 2026 – Supplemental Response to Public Comments Heard at the September 16, 2025 Public Meeting
Appendix D - Cambium Technical Response to Public Comments dated March 11, 2026
Appendix E - Cambium Letter Addendum - TW3 Pumping Test dated March 11, 2026

Prepared by:

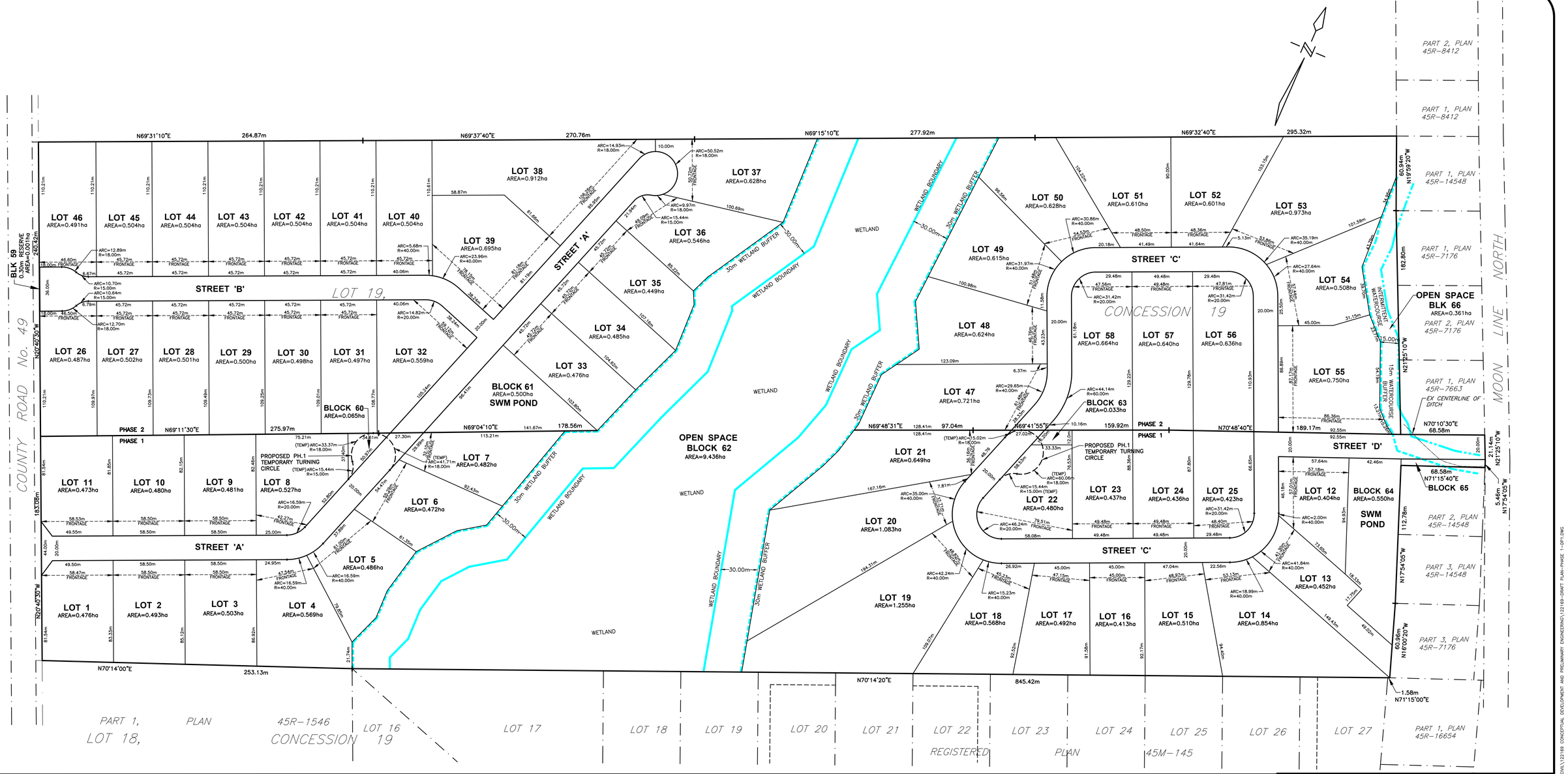
Adele Arbour, MCIP, RPP
Planner

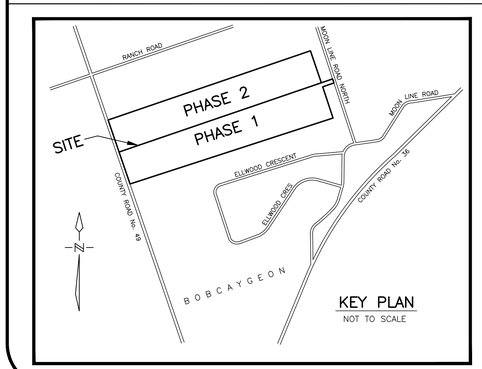
Approved by:

Donna Teggart,
CAO/Treasurer

Submitted by:

Eric Guay,
Director of Building &
Planning/CBO





LAND USE SCHEDULE

PROPOSED USE	LOT/BLK #	# OF LOTS/BLKS	# OF UNITS	AREA (ha)
LOW DENSITY RESIDENTIAL				
SINGLE DETACHED	LOTS 1-58	58	58	33.204
NON RESIDENTIAL				
RESERVES	BLOCK 59	1		0.001
ROAD WIDENING	BLOCK 65	1		0.041
FIRE FIGHTING PROTECTION	BLOCK 60, 63	2		0.098
OPEN SPACE	BLOCK 62, 66	2		9.973
SWM PONDS	BLOCK 61, 64	2		1.042
ROADS	20.0m R.O.W.			4.181
TOTALS		65	58	48.540

PLANSCAPE
BUILDING COMMUNITY THROUGH PLANNING

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51 OF THE PLANNING ACT

F NORTH – OPEN SPACE
SOUTH – ESTATE RESIDENTIAL
EAST – ESTATE RESIDENTIAL
WEST – ESTATE RESIDENTIAL
H – WELL AND SEPTIC
I – TILL
K – PRIVATE WELL

OWNER'S AUTHORIZATION

I/WE **LAND OWNER**

BEING THE REGISTERED OWNER OF THE SUBJECT LANDS HEREBY AUTHORIZE

PLANSCAPE

TO PREPARE AND SUBMIT A DRAFT PLAN OF SUBDIVISION FOR APPROVAL

SIGNED *ORIGINALLY SIGNED BY: SCOTT JEFFERY*

TITLE

DATE *DECEMBER 21, 2022*

SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THE BOUNDARY OF THE LANDS TO BE SUBDIVIDED AS SHOWN ON THIS PLAN AND THEIR RELATIONSHIP TO ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN

ONTARIO LAND SURVEYOR

ONTARIO LAND SURVEYORS

SIGNED *ORIGINALLY SIGNED BY: MARYNA A. HANNA*

O.L.S

DATE *SEPTEMBER 2, 2022*

No.	REVISION	DATE	BY	APPROVED
4.	REVISED TO INCLUDE PHASE 2 LANDS	06/26/2025	JG	
3.	ADDED BLOCK 31 (SWM) & RENUMBERED LOTS	12/19/2024	CJ	
2.	REVISED LOTS 12 & 13, ADDED BLOCK 30 (SWM)	12/03/2024	BB	
1.	REVISED AS PER 1ST SUBMISSION COMMENTS	03/18/2024	MH	

REVISIONS

PROPOSED

DRAFT PLAN

PART OF LOT 19, CONCESSION 19
FORMERLY IN GEOGRAPHIC TOWNSHIP OF HARVEY
NOW IN THE
MUNICIPALITY OF TRENT LAKES
COUNTY OF PETERBOROUGH

D.G. BIDDLE & ASSOCIATES
CONSULTING ENGINEERS & PLANNERS
481 Taunton Rd W, Oshawa ON
150 King St, Peterborough ON
Phone: 905-576-8500
info@dgbiddle.com
dgbiddle.com

SCALE: 1:1500
DRAWN BY: B.B.
DESIGN BY: M.J.H.
CHECKED BY: M.B.C.
PLOT DATE: 02/07/2025

122169

DP-1

X:\SHP\JOB FILES\122000\122169 JEFFERY - BOBCAYGEON\122169 DRAWINGS\122169 CIVIL\122169 CONCEPTUAL DEVELOPMENT AND PRELIMINARY ENGINEERING\122169-DRAFT PLAN-PHASE 1-DP1.DWG

760 Peterborough County Road 36, Trent Lakes, ON K0M 1A0 * Ph 705-738-3800 Fax 705-738-3801

January 14, 2026

Planscape Inc.
104 Kimberley Avenue
Bracebridge ON
P1L 1Z8

Re: Supplementary Response to Comments Heard at the September 16, 2025 Public Meeting

Attention: Debbie Vandenakker

Thank you for providing the supplemental memo December 3, 2025 with respect to the applications for Official Plan, Zoning By-law Amendment and Plan of Subdivision for lands located at 168 County Road 49 in Part Lot 19, Concession 19 (Harvey). Trent Lakes planning staff have had an opportunity to review and discuss this additional information with planning staff from the County of Peterborough and the following is a summary of our comments and direction at this time:

1. Please add a date and signature to the submission.
2. We would suggest the themes of concern be listed on Page 1 after the third paragraph (or see summary of comments noted on Page 2 of this letter).
3. In the summary addressing MDS it should be noted that there are 4 barns in the vicinity of the development and commentary should be provided with respect the 4th barn.
4. County and Municipal staff will be requesting the Hydrogeological peer review consultant to review items in your submission, addressing the feature-based water balance, well water quantity, well water fracking and the Reasonable Use Assessment to confirm they are in agreement with the supplemental and/or revised submission. On this basis it may be prudent to submit the original report or memo prepared by your hydrogeologist that was used as the basis for the supplemental memo submission.
5. The memo states that the existing supply well is denoted as MW2 in Figure 1. Figure 1 illustrates 4 monitoring wells but none are denoted as MW2. Does this paragraph of the report relate to a different figure?
6. The second paragraph of page 6 may have a typo in the reference to an increase in the flow rate to 10 L/min.
7. Figure 1 does not appear to illustrate the location of the new test well denoted as TW3.

8. The summary of the pump test data appears to conclude the test wells can sustain yields in compliance with Procedure D-5-5, with the exception of TW2 where storage is recommended. Given that one of the concerns that was raised by neighboring landowners is the potential impacts upon neighbouring wells, staff would suggest some further analysis or commentary be provided to specifically address impacts to existing wells proximate to the proposed development.
9. It would appear that the proposed inclusion of ARUs is creating additional potential demands on water and septic servicing and it may help to mitigate this issue if ARUs were not permitted at this time.

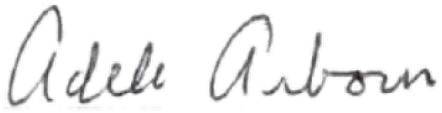
Staff have also reviewed the meeting minutes and notes and the following is our summary of issues and comments that were raised:

- The barn located at 214 County Road 49;
- The potential impact of the development on speed limits on County Road 49;
- Spreading of sewage on neighbouring lands;
- Does hydrogeological study pertain only to the subject property or does it take into consideration the surrounding area;
- Drainage ditches and culverts may not be big enough;
- Why does the entrance off of Moon Line Road go through a wetland/watercourse;
- Accessory dwelling units could double the residential occupancy;
- Stormwater currently goes south to adjacent property;
- Will there be fencing around the subdivision;
- Why is 1,000 litres per day used as the basis for nitrate attenuation;
- Would hydrofracking be required;
- There is a plan of subdivision proposed nearby in Kawartha Lakes is it on private services or municipal services; and,
- Is there another development in the County which has 100 homes with 100 wells.

In order to ensure the supplemental submission responds directly to questions and comments raised at the public meeting, staff would suggest the preparation of a table that lists the above comments together a response, which in most cases can be referenced from the body of the supplemental memo.

If you would like to meet to review our comments please feel free to call me.

Yours truly,



Adele Arbour, RPP MCIP
Planner

Supplementary Responses to Comments Heard at the September 16, 2025 Public Meeting

Project: Jeffery Homes Subdivision
Address: 168 County Road 49
Preparation Date: December 12, 2025 with additional information provided January 18, 2026 and February 4, 2026 at the request of Township Staff
Prepared By: Planscape Inc., agent of Jeffery Homes with support from Technical Consulting Team

Purpose of this Communication:

At the statutory public meeting for the Official Plan Amendment and Zoning By-law amendment for the residential development at 168 County Road 49 on September 16, 2025 there were several questions raised by attendees at the meeting. To ensure the surrounding neighbourhood and any interested members of the public are well informed about the technical details of this project, this communication has been prepared.

We heard clearly several concerns about water quantity for wells south of the project site. We understand that some wells were currently experiencing low volume. To give residents and the municipality a greater level of confidence, an additional well was installed and pump tested and two other wells were re-tested to be able to provide 2025 data for concerned residents. This work is in addition to the hydrogeological work already conducted and peer reviewed as part of the approvals process for this project.

Themes of concern were grouped together as follows and responses provided:

- Minimum Distance Separation Formula (MDS)
- Well Water Levels
- Feature Based Water Balance
- Well Water Quantity
- Well Water Fracking
- Stormwater Run-Off in the South Portion of Wetland
- Septic Calculations and Additional Residential Units
- Traffic Impacts
- Entrance off Moonline Road
- Additional Residential Units (ARUs) and Servicing Concerns

Overview of the Proposed Development:

A Plan of Subdivision is proposed on a property which is approximately 48.15 hectares in size to create 59 total residential lots. An unevaluated wetland and associated buffer occupy approximately 9.80 hectares of the subject property, leaving 38.35 hectares of the property developable. The proposed lots range in size from 0.4 hectares in size to 1.3 hectares. This development is divided into two phases, with the first creating 25 new lots and the second creating 33.

In support of the proposed development, the following studies were undertaken to ensure the appropriateness of the proposed development:

- Archaeological Assessment
- Environmental Impact Study
- Hydrogeological Assessment Report
- Geotechnical Study
- Planning Justification Report
- Functional Servicing and Stormwater Management Report
- Servicing Plans and Pre and Post Development Drainage and Erosion and Sediment Control Plans
- Transportation Impact Study
- Minimum Distance Separation Report

Minimum Distance Separation Formula (MDS)

In Ontario's rural and prime agricultural areas, the Provincial Planning Statement, 2024, and other provincial land use plans require that new land uses, including the creation of lots and new or expanding livestock facilities, comply with the Minimum Distance Separation (MDS) formulae and guidelines.

The MDS formulae and guidelines are land use planning tools that determine setback distances between livestock barns, manure storages or anaerobic digesters and surrounding land uses, with the objective of minimizing land use conflicts and nuisance complaints related to odour. Demonstrating that an MDS setback can be met, may be required before a land use planning approval or building permit can be issued.

MDS is comprised of two separate, but related, formulae that act reciprocally to help site either new development or new/expanding livestock facilities:

> MDS I setbacks are determined between proposed new development and existing livestock barns, manure storages and/or anaerobic digesters.
>
> MDS II setbacks are determined between proposed new or altered livestock facilities and/or anaerobic digesters and existing or approved development, lot lines and road allowances.

Phase 1 of the subdivision is located within an approved settlement area; therefore, the MDS Formulae does not apply. An MDS I calculation for Phase 2 was prepared with the results provided in the Planning Justification Report. Barns in the vicinity of the subject property were reviewed to assess the presence of livestock, manure storage, or anaerobic digesters and to determine their respective influence areas.

There was one barn of interest in the MDS exercise and investigation. Since the time of the public meeting, an application from that landowner for an Environmental Compliance Certificate for approval to spread sewage has been received by the province. Through that review process, the Township has requested that the zoning on that property be changed to reflect the current commercial activities. Jeffery Homes has also provided a comment to the province supporting the Township’s recommendation.

Well Water Levels

Residential dwellings, wells, and septic systems will be constructed on each lot. Lots are sized to accommodate wells and septic systems on the same lot and a hydrogeological study has been undertaken and submitted as part of the complete application for this development.

The hydrogeological investigation and terrain analysis dated October 2021 was undertaken by Jp2g which assessed the water supply and septic system suitability for the proposed development based on private servicing. The report was prepared following the Ministries MOE, MOEE, MOECC, MECP Hydrogeological Technical Information Requirements for Land Development Applications. Geology maps, surrounding land uses, water well records of nearby residences, drilling and testing of four on-site test wells, and a complete terrain analysis were undertaken. Hydrogeologic information was taken from the test well information and from water well records of nearby wells. Surrounding wells confirmed the varying depth of wells and anticipated well yields, and all indicated the provision of a fresh water supply.

Cambium Inc. (Cambium) completed a Hydrogeological Assessment and Feature Based Water Balance report, dated July 23, 2025, in support of the proposed development. This assessment characterized the soil and groundwater conditions at the site, assessed the pre- and post-development water balance to determine potential groundwater control requirements during construction, and evaluated potential impacts on the surrounding natural environment as a result of the proposed development. The subsurface conditions consisted of topsoil underlain by glacial till containing a mix of sand, gravel, silt, and some cobbles. Groundwater levels in the four on-site monitoring wells were measured between 0.12 and 4.38 metres below the ground surface, corresponding to elevations between 283.50 and 299.46 metres above sea level. Monitoring was conducted on November 10, 2023, and March 13, April 17, May 28, June 17, and July 8, 2025. Over this period, groundwater levels in the piezometers ranged from 0.38 metres above to 1.58 metres below the ground surface, with corresponding elevations between 287.97 and 289.76 metres above sea level. Horizontal groundwater flow was directed toward the southeast, while vertical flow gradients in the wetland fluctuated between upward and downward depending on seasonal conditions, indicating that portions of the wetland are groundwater-fed at certain times of the year. The pre- and post-development water balance identified an infiltration deficit of approximately 19,600 m^3 per year (8,600 m^3 for Phase 1 and 11,000 m^3 for Phase 2). To maintain pre-development infiltration rates, the report recommended low impact development (LID) measures such as roof downspout disconnections and three infiltration galleries designed by D.G. Biddle and Associates (Biddle). The nitrate assessment determined that wastewater from the proposed 59 estate lots would result in a nitrate concentration of 8.14 mg/L at the property boundary, below the Ontario Drinking Water Quality Standard of 10 mg/L. Dewatering may be required during construction where infrastructure intercepts the groundwater table, and setback requirements prescribed by Kawartha Conservation will be maintained to protect the on-site wetland and natural heritage features.

The hydro-geological study was undertaken as per provincial guidance. It inherently takes into consideration surrounding lands as it determines if there is acceptable quantity and recharge on site to support the proposed wells. By concluding that there is sufficient quantity and recharge, the study indicates that no impacts are expected to surrounding wells.

Feature-Based Water Balance

A feature-based water balance is important to evaluate the potential development impacts on a wetland feature and to ensure that the detailed stormwater management design will maintain similar levels of surface water runoff to the wetland as well as not significantly disrupt the existing hydroperiod of the wetland. The subject property has a mapped unevaluated wetland which occupies much of the central and northeast portions of the property. The requirement for a feature-based water balance is prompted when a proposed development has the potential to alter the wetland catchment's water balance either by adding or removing flow to the wetland outside pre-development conditions. Phase 1 of the proposed development is located entirely outside of Wetland 1 and the 30-metre setback, so no alterations to flow patterns are expected post-development. Therefore, a feature-based water balance would not be required for Phase 1 of the development.

The feature-based water balance is underway and anticipated to complete in March 2026 to better understand the hydraulic capacity of and inform site-specific mitigation strategies for the wetland. This work does not impact the Draft plan proposal of 58 lots, rather it will help protect the wetland function through mitigation measures.

The methodology for a feature-based water balance includes determining the function of the wetland as being a groundwater recharge or groundwater discharge feature. This is currently being assessed at the property. Cambium has installed three well nests in Wetland 1. The well nests consisted of shallow and deep groundwater wells, which groundwater levels are currently being measured from on a monthly basis. Using this data, the direction of vertical groundwater flow (vertical gradients) in the wetland will be determined. At the conclusion of one year of monitoring, the wetland will be determined as a recharge or discharge feature. As mentioned previously, the intent of the year long study of the water levels within the wetland is to determine its seasonal hydroperiod. Following this, the model assesses the variability in water levels monthly and seasonally to calculate the range of sensitivity and significance of the hydroperiod. Once the model is created for the wetland, it can be assessed and compared to the post-development water balance, including any stormwater discharge, to determine if any impacts may occur or possible methods for mitigation.

Well Water Quantity

The proposed development will be serviced for water and wastewater on-site. In Ontario, the Ministry of Environment Conservation and Parks (MECP) Procedure D-5-5 Technical Guideline for Private Wells: Water Supply Assessment (MECP, 1996) provides technical guidance to professionals in land development of the assessment of groundwater quality and quantity. Jp2g completed a water supply assessment as per Procedure D-5-5 in the Hydrogeological Investigation and Terrain Analysis Report. The Jp2g report was reviewed and referenced for information to answer this concern.

With respect to water quantity, Procedure D-5-5 states that each future domestic well must provide sufficient water for normal domestic purposes as determined from running pumping tests in test wells. There were 4 test wells that were advanced into the underlying limestone/dolomite bedrock aquifer at the subject property. The depths of the wells ranged from

30 to 55 metres below ground surface. Two of the supply wells were advanced on the west side of the wetland, and two wells were advanced on the east side of Wetland 1.

As per the Jp2g report tests were conducted on each of the 4 test wells at a pumping rate of 13.6 litres per minute (L/min) for 6 hours. The rate of 13.6 L/min is the minimum pumping rate for a single detached dwelling as per Procedure D-5-5. When one test well was being pumped, the others were used as monitoring wells to monitor water levels interference from the pumping well.

Minimal drawdowns were recorded for the test wells ranging between 1.13 and 1.78 m, with Jp2g noting that the test wells did not report well interference effects on the non pumping wells while pumping the respective test wells. The available drawdown in the pumping wells ranged between 16.61 and 30.25 m at the end of each test, indicating ample water remaining in each well. Each test well recovered to 95% of pre-pumping static water levels in the test wells within 5 to 120 minutes, indicating a robust aquifer.

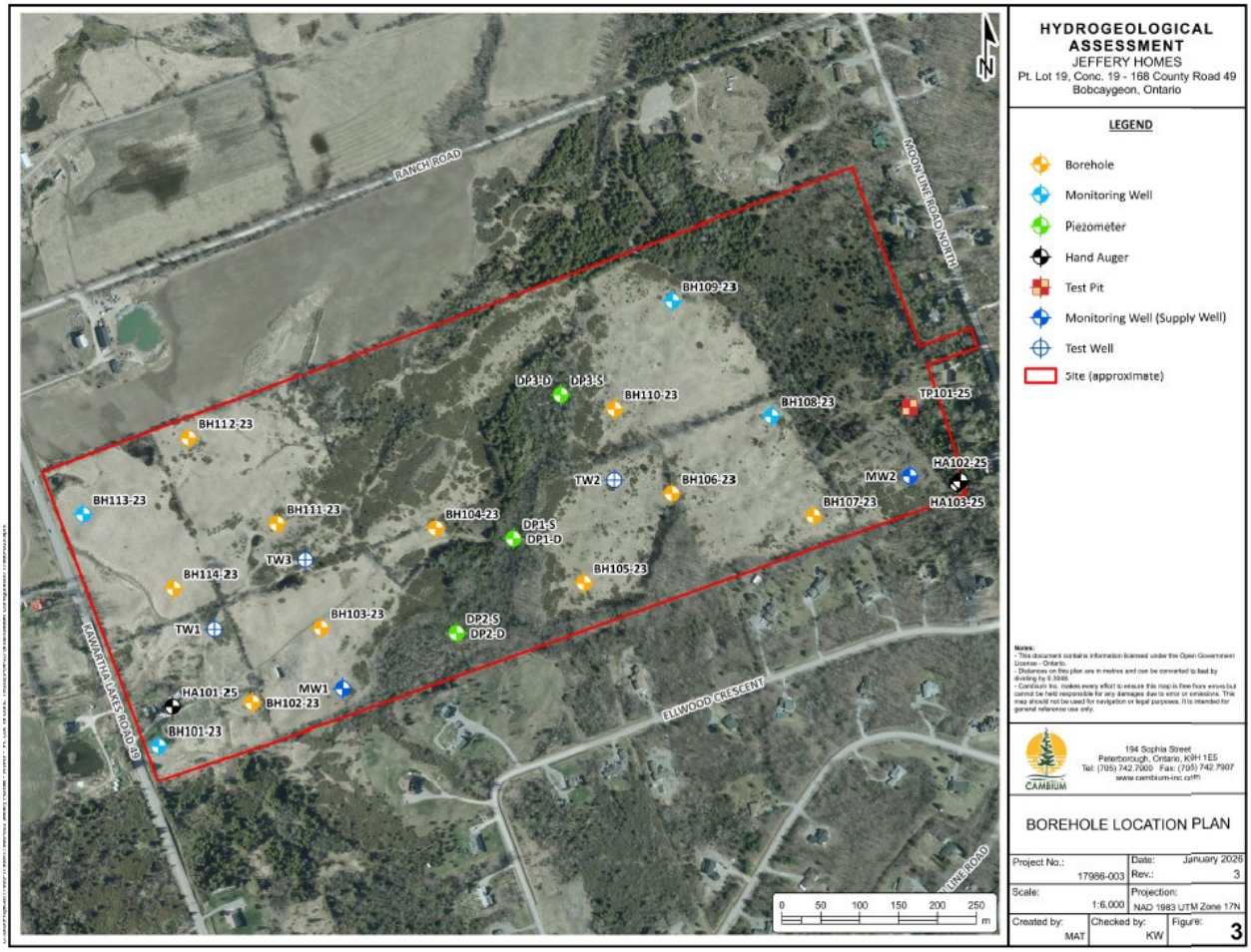
Additionally, Jp2g calculated the limestone bedrock transmissivities (aquifer's capacity to transmit water) for three of the test wells. The calculated transmissivity was considered high at 44.1 to 142.9 m^2/day. Considering the high transmissivity, the fast recovery rates, and low to minor interference effects, Jp2g concluded that an adequate supply of groundwater from the bedrock aquifer is available for Phase 1 of the development.

It is Cambium's opinion that based strictly on the pumping tests data and water supply assessment presented for Phase 1, the work of Jp2g adheres to Procedure D-5-5 and the water supply can be adequately sourced without significant off site well interference.

Cambium completed supplemental hydraulic tests to validate the Jp2g water supply assessment results at the Site. Cambium returned to Site November 4 and 5, 2025, to hydraulically test two of the existing four wells on-site to confirm the conclusions outlined by J2Pg.

One well was tested on each of the wetlands. On the west side of the wetland, the existing well located on proposed Lot 9 (Well ID A104942) was selected to test and denoted as TW1. The existing supply well located on proposed Lot 5 (A104945) was used as a monitoring well and denoted as MW1. On the east side of the wetland, the existing supply well located on the proposed Lot 20 (A104944) was selected to test and denoted as TW2 during the hydraulic pumping test. The existing supply well located on proposed Lot 13 (A104943) was denoted as MW2. The location of the test and monitoring wells are presented on Figure 1.

Figure 1: Borehole Location Plan



The pumping test for TW1 was conducted on November 4, 2025. The static water level prior to testing was 13.41 mbgs. The test commenced at 9:30 and ran for a continuous 3 hours. A constant rate of 20 L/min was maintained for the duration of the test, with the exception of the flow rate being accidentally lowered to 14 L/min at 10:15 but was adjusted back to 20 L/min by 10:20. The pump was shut off at 12:40. A drawdown of 4.54 m was observed at the end of the 3 hours. The available drawdown at the end of the test was 23.12 m. The test well recovered to 95 % of pre-pumping static levels within 4 hours of the pump shut off. Water level were monitored to MW1 throughout the hydraulic testing on TW1. There was no measurable response in MW1.

The TW2 pumping test was conducted on November 5, 2025. The static water level prior to testing was 8.04 mbgs. The test commenced at 7:30 and ran for a continuous 3 hours. A constant rate of 20 L/min was initially set for the pumping rate before the flow rate was lowered to 14 L/min due to a substantial increase in drawdown. The flow rate was further increased to 10 L/min at the 87-minute mark to test the substantial rate of the supply well. The rate was reduced back to 8 L/min from the 127-minute mark and maintained to the end of the test. The pump was shut off at 10:30.

A maximum drawdown of 20.82 m during the test was observed before the flow rate was lowered to 8 L/min for the first time. After the flow rate was increased to 10 L/min and then was

reduced again to 8 L/min, recovery was observed during the test which continued into the recovery period after the pump was shut off. TW2 recovered to 81% of pre-pumping static-level within 1 hour of the pump shut off. Water levels were monitored in MW2 throughout the hydraulic testing on TW2. There was no measurable response in MW2.

The results of the testing completed at wells TW1 and TW2 confirm the previous results (note – TW2 was the lowest yielding of the four tested well in the Jp2g study) and the conclusions of the Jp2g report. The results indicate that these wells can provide the minimum yield, on a daily sustainable basis, as outlined by Procedure D-5-5. The hydraulic testing at TW1 indicated that the well had no issues with supplying water at a rate of 20 L/min, with a 95% recovery within 4 hours of the test ending, without inducing a measurable response in the closed monitoring well MW1. The hydraulic testing at TW2 indicated that the well was able to sustainably supply water at a rate of 8 L/min, with 81% recovery within 1 hour of the test ending. While 8 L/min is the steady pumping rate the well can produce in perpetuity, and this is below the 13.7 L/min requirement of Procedure D-5-5, this well will be able to support a higher pumping rate for shorter pumping intervals, which is more typical of water usage in a residence. It is noted that the regular well is not continuous, and there was fast recovery in the well. If required, storage can be added for this well to accommodate peak demand usage.

Pumping tests of TW1 and TW2 were completed on-site to validate the prior water supply assessment of Jp2g, knowing one ARU is being proposed for each lot. The testing of well TW1 completed at a higher pumping rate of 20L/min compared to 13.7 L/min from the Jp2g investigation to account for the ARU.

The testing showed that TW1 could supply water at a rate of 20 L/min, over 3 hours with a total volume discharged of 3,750 L which. This was considered adequate for 2,740 L demand for a residence with an ARU.

The testing of well TW2 indicated that the well had no issues with supplying water at 8 L/min. While this rate is under the 13.7 L/min requirement for Procedure D-5-5, the well will be able to support higher pumping rates for shorter intervals. Over 3 hours, a volume of 1770 L was pumped. Additional storage can be provided to accommodate the higher demand of a residence including the ARU of 2740 L.

Additionally, the Client installed one new test well on the north (Phase II) side of the Site to meet the test well requirement of Procedure D-5-5, for Phase II submission. The new well was denoted as TW3. The hydraulic test for well TW3 discharged a total volume of 9,450 L, which far exceeds the estimated 2,740 L water demand required for a 4 bedroom house including an ARU. Since the well can provide much higher yield on a daily basis that the minimum yield outlined by Procedure D-5-5 and there were no significant influences for water supply for the proposed residential dwelling including the ARU. Over the 3 hours of pumping TW1 and TW2, approximately 3,750 L and 1,770 L respectively was discharged from the wells. TW1 had ample available drawdown of 23.13m of the end of the test, and TW2 had 3.7 m of available drawdown at the end of the test and notably had fast recovery. Overall, all of the tested wells except TW2 can support the higher pumping demand of a residence with an ARU of 2,740 L and storage can be added to well TW2 to accommodate peak demand usage.

Well Water Fracking

Water well fracking or hydrofracking is a process where high-pressure water is surged into bedrock fractures to enlarge them and also to flush debris from the fractures. This procedure is often done to increase water production in low-yielding wells.

It should be noted that all four of the test wells on-site had adequate water yield and did not require hydrofracking. The well records from the test wells on-site had recommended pumping rates ranging between 13.6 and 37.9 L/min.

A review of the MECP Water Well Information System (WWIS) indicated a total of 73 water supply records within 500m of the subject property. Of the 73 supply well records, 71 records were installed into the limestone bedrock aquifer and only 2 of them were installed into overburden. Of all the well records, 89% reported a recommended pumping rate above the 13.6 L/min (or 3 imperial g/min) minimum yield rate for a private water well of Procedure D-5-5. Cambium notes that there are eight well records (11% of the supply wells) that have a recommended pumping rate of under 13.6 L/min (5 and 9 L/min). This indicates that the bedrock aquifer yield is not uniform throughout the region. However, even at these reduced pumping rates, a well could meet the daily water demand for a typical four-bedroom dwelling with between 4.2 and 7.5 hours of operation without requiring well hydrofracking. As such, based on the hydrogeological conditions tested on the subject property, hydrofracking is not anticipated for any wells drilled on the property.

Stormwater Run-Off in the South Portion of Wetland

A comparison of the Pre-Development Storm Drainage Plan – East, Drawing SD-2 (D.G. Biddle & Associates, July 31, 2025) and the Post-Development Storm Drainage Plan, Drawing SD-4 (September 25, 2025) confirms that the contributing drainage area along the rear lot line of lot 19, which is adjacent to existing lots 20 and 21 to the south, will be moderately reduced in post development conditions. As a result, the stormwater volumes and peak flows will also be reduced compared to what is flowing there currently.

Further, it is understood that a registered drainage easement exists in favour of the Township on the properties south of proposed Lot 18 and 19. This easement legally supports the continued use of the outlet for drainage purposes. The outlet location remains unchanged, and the flow is expected to be less than under pre-development conditions. We will ensure any discharge offsite will be directed towards the registered drainage easements.

Septic Calculations and Additional Residential Units

The daily sewage flow volume of 1,000 L/day used in the nitrate attenuation assessment is the average daily sewage effluent flow volume outlined in MECP Procedure D-5-4 which was followed in the preparing of the calculations. Conversely, a rate of 2,000 L/day is used for a 4 bedroom house under the Ontario Building Code to quantify the peak daily sewage flow rate for adequate sizing in the design of a septic system. Septic systems at the subject property will be required to be designed based on the maximum daily sewage flows referenced in Part 8 of the OBC. For hydrogeological assessment for rural subdivision developments, the average sewage

flow is representative of the typical daily flow for the subdivision lots in order to assess the nitrate contribution to the receiving aquifer system.

Cambium completed preliminary calculations to confirm feasibility of ARUs. Cambium made the assumption for the nitrate calculations that each lot would have 1 ARU (58 ARUs total) at the flow rate specified (490 L/day per ARU). It is noted that an ARU can be considered an in-law suite or a detached building, as per Part 8 of the Ontario Building Cod, which would have different sewage design flows. Our calculations show that 58 lots each with an ARUs (sewage flow of 490 L/day) is feasible without exceeding a nitrate concentration limit of 10 mg/L at the property boundary.

Cambium revised the nitrate attenuation calculations to include the Wetland 1 area from the continued monitoring at the subject property, because it is known that shallow groundwater flows through Wetland 1, and thus can be considered as dilution area for the assessment of nitrate. The available dilution area was determined to be 476,000m^2 which is the difference between the site area (481,000m^2) and the stormwater pond (5,500 m^2).

A sewage flow of 1,450 L/day was estimated based on the following:

- 1,000 L/day for the proposed single detached dwelling (Guideline D-5-4)
- 450 L/day for the additional residential unit (calculated using the flow equivalent to a 1-bedroom apartment – i.e., 1.4 persons per unit at 350 L/p/d = 490 L/day per ARU).

Guideline D-5-4 assumes an average usage while septic systems are designed based on maximum peak usage under the OBC.

Total nitrogen (all species) ultimately converts to nitrate through the wastewater treatment process. Nitrate is considered to be the critical contaminant in sewage effluent. A nitrate loading of 40 mg/L is used to determine the domestic effluent loading from conventional septic systems on the receiving subsurface. To determine the concentration of nitrate at the property boundary, a mass balance calculation was completed.

The nitrate concentration at the property boundary is estimated at 6.84 mg/L using conventional wastewater treatment for 59 single detached residential dwellings, or 9.38mg/L for 59 single detached residential dwellings each with an ARU. Therefore, the subject property could be developed to accommodate the proposed 59 lots, including up to 59 ARUs without exceeding a nitrate concentration limit of 10 mg/L at the property boundary.

Traffic Impacts

A Transportation Impact Assessment was undertaken in April of 2022 by Greer Galloway and peer reviewed. The following Traffic Impact Study was undertaken by NexTrans in February of 2024 which addressed the peer review of the previously submitted Transportation Impact Assessment.

The Greer Galloway Traffic Impact Assessment was undertaken to assess the potential effects of traffic resulting from the proposed development and to identify any roadway improvements required to ensure that roadways will continue operating at an acceptable level of service upon the completion of the development.

This Assessment followed the guidelines of the MTO General Guidelines for the Preparation of Traffic Impact Studies utilizing Highway Capacity Software (HCS7) to determine if the existing road network has the ability to service the proposed development at an acceptable level of service. The traffic volumes generated by the proposed development were estimated using the Institute of Transportation Engineers (ITE) Trip Generation, 10th Edition, Code 210. Any values that could not be obtained due to lack of data were assumed using conservative values. Pedestrian traffic was not considered since there are no pedestrian crossings on the existing road network and pedestrian traffic is not known to be significant.

Analysis of various peak hour time periods (Weekday AM, Weekday PM) was conducted to determine the worst-case scenario. The worst-case scenario was then analyzed using a horizon time of 10 years (to year 2032) to analyze the capacity considering background traffic growth conditions.

Based on trip generation calculations, the proposed development is expected to generate a total of 49 two-way gross trips (12 inbound and 37 outbound) during AM peak hour, and a total of 63 two-way gross trips (40 inbound and 23 outbound) during PM peak hour.

Under future total traffic conditions, all movements of the study area intersections analyzed, as well as the proposed site access onto County Road 49 and Moon Line Road North are projected to operate with residual capacity, with excellent levels of service, and with manageable delay and queue lengths during both AM and PM peak hour.

Road standards, speed limits and safety concerns are not within the ability of the development to influence. County staff should be contacted directly to address these concerns. Similarly, concerns regarding the culverts and ditches should be directed to County staff. It is a requirement of the development that water from the site must meet pre-development run-off and infiltration rates and therefore not impact the receiving ditches.

Entrance off Moon Line Road

A question was raised regarding why the entrance on Moon Line Road traverses a watercourse. As per the EIS, this watercourse does not meet the standards of protection. A culvert to maintain water flow will be installed to conduct the tributary water.

Additional Residential Units and Servicing Concerns

It is predicted that between 6 and 12 lots of the 59 total lots in this development may wish to add an ARU. As ARUs are a “new” concept in terms of providing increased permissions in various land use designations, the prediction comes from a variety of public information sources and was generated by Microsoft Co-Pilot. The project undertook this predictive analysis as there is no provincially reported statistics that can be relied upon due to the recent inclusion of ARU policy in Ontario’s land use planning framework.

Co-pilot describes the process used to generate the prediction as follows: Microsoft Copilot is an AI assistant built into our Microsoft tools that helps staff quickly gather and summarize information from credible, publicly available sources. It does not make decisions — it helps us understand the policy landscape more efficiently. The ARU uptake prediction provided here is

based entirely on Ontario legislation and municipal reports, especially the fact that provincial ARU permissions do not apply to rural lots on private servicing. Copilot helped organize and analyze these sources, and planning staff reviewed the output to ensure it reflects real-world conditions. The result is a transparent, evidence-informed estimate supported by clear references.

Please note that the County is in the approval process of a Township Official Plan and Zoning By-law Amendment that will allow 1 Additional Residential Unit (ARU) per Rural Residential lot, subject to meeting certain criteria, as of right. The proposed lot sizes of this application far exceed the required lot sizes for ARUs in the Official Plan and Zoning By-law amendment for approval with the County.

Sources used:

- Association of Municipalities of Ontario (AMO). *Submission to the Ministry of Municipal Affairs and Housing Regarding Proposed Amendments to O. Reg. 299/19: Additional Residential Units (ERO 019-9210)*, October 23, 2024. [london.ca]
- City of London. *Additional Residential Units Amendments as a Result of the More Homes Built Faster Act, 2022 (Bill 23)*. Report to Planning & Environment Committee, May 23, 2023. [ero.ontario.ca]
- City of Toronto. *City Comments on Proposed Amendment to Ontario Regulation 299/19 – Additional Residential Units*, Report for Action, October 21, 2024. [amo.on.ca]
- Environmental Registry of Ontario. *Proposed Changes to Ontario Regulation 299/19: Additional Residential Units (ERO 019-6197)*, Decision Posted January 6, 2023. Ministry of Municipal Affairs and Housing.
- Municipality of Lakeshore. *Additional Residential Units (ARUs) – Information Page*. [havnhome.com]
- Town of Orangeville. *Additional Residential Units (ARU) & 2024 Final Building Permit Report*, Report INS-2025-005, February 10, 2025. [lakeshore.ca]
- City of Stratford. *Additional Residential Units Pamphlet*, Building and Planning Department, 2022. [pub-orange...etings.com]

There was a statement made at the public meeting that the ARUs could double the residential occupancy. However, the ARU permissions are for 1-bedroom units and as per the prediction above, the estimate is between 6 to 12 additional bedroom units total for the entire development. Water testing and wastewater flow estimates show that 58 ARUs could be supported, but the expectation is that far less than that number will actually be applied in this development.

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March 11, 2026

Jeffery Homes
1200 Airport Blvd, Suite 201
Oshawa, ON L1J 8P5

Attn: Scott Jeffery
Owner

Re: Technical Response to Public Comments for Part of Lot 19, Concession 19, Municipality of Trent Lakes, County of Peterborough
Cambium Reference: 17986-003

Dear Scott Jeffery,

Jeffery Homes (Client) retained Cambium Inc. (Cambium) to complete a Hydrogeological Assessment and a Feature Based Water Balance to support the proposed development at Part Lot 19, Concession 19, Municipality of Trent Lakes (previously known as Township of Galway-Cavendish, County of Peterborough (Site). It is understood that the Client is pursuing draft plan approval from the Municipality of Trent Lakes (Municipality) for two phases of a residential subdivision (herein referred to as Phases I and II) of a proposed residential development.

A meeting was held on December 2, 2024 with the project team and Kawartha Conservation (KC) to discuss the proposed development. Through this meeting, a staged approach for approvals (i.e., Phase I proceeding in advance of Phase II) was agreed upon by KC. The phased approval approach is in acknowledgement that potential impacts to wetlands within the Phase II lands, as well as outstanding peer review comments, will need to be addressed to facilitate Phase II approval.

A public council meeting was recently held on Tuesday September 16, 2025 which brought forward public comments; as a result of the meeting, the Municipality wants responses provided prior to draft plan approval. The objective of this letter report is to provide response to these concerns for the Municipality to review and support for the next public council meeting. It is noted that some of the public comments relate to the water supply assessment for the Site.

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Cambium was not retained to develop a water supply assessment for the Site. The water supply and wastewater assessment for Phase 1 of the development was included in the *Hydrogeological Investigation and Terrain Analysis* report completed by Jp2g Consultants Inc. (Jp2g) dated October 2021. Cambium has reviewed this report and will include reference to it where applicable. Note that Cambium's review is cursory and cannot confirm details of fieldwork and field data preformed as part of this investigation.

SITE DESCRIPTION

Cambium understands the proposed development includes the construction of 58 estate lots, with 25 lots planned for Phase 1 of development and 33 lots planned for Phase II. The property consists of approximately 48.15 ha of undeveloped land, except for an existing dwelling and associated structures in the westernmost area bordering County Road 49. The Site is proposed to be serviced privately for water supply and wastewater.

The Site is situated in a Kawartha Conservation (KC) regulated area as well as a Natural Heritage System Area mapped on the Site. An unevaluated wetland and wetland buffer occupy about 9.80 ha of the Site, leaving about 38.35 ha as the developable area. As outlined in the Environmental Impact Study (EIS) (Cambium, 2024), three wetlands were identified and delineated on the Site. Of these, two are proposed for removal (Wetlands 2 and 3). Wetland 1 will be retained, will be appropriately buffered, and will serve to maintain the ecological function of the Site.

HYDROGEOLOGICAL ASSESSMENT SUMMARY

The purpose of Cambium's Hydrogeological Assessment (dated July 23, 2025) was to characterize the soil and groundwater conditions at the Site, assess the pre- and post-development water balance, to discuss the need for groundwater control during the construction process, and to assess any impacts on the surrounding natural environment due to the proposed development. A brief description of the conclusions are summarized below:

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- Cambium's borehole investigation showed soils on-site were generally topsoil overlaying a glacial till soil with a relatively even mixture of sand, gravel, and silt, and some cobbles.
- Groundwater levels in the four monitoring wells on-site were measured between 0.12 to 4.38 metres below ground surface (mbgs) with groundwater elevations ranging between 283.50 to 299.46 m above sea level. The monitoring period for manually measured water levels included November 10, 2023, March 13, April 17, May 28, June 17, and July 8, 2025.
- Over the monitoring period the groundwater levels in the nested piezometers ranged in depth from 0.38 m above ground surface to 1.58 mbgs, while the elevations ranged from 287.97 to 289.76 masl.
- Horizontal shallow groundwater flow direction on-site was determined to be towards the southeast.
- Vertical groundwater flow gradients in the wetland indicated fluctuating gradients between well nests DP2 and DP3 and a slight downward gradient at well nest DP1. The upward gradient results in both DP2 and DP3 stations indicate that the wetland in these areas are likely groundwater fed at least part of the year.
- The water balance indicated that there would be an infiltration deficit upon development of the Site in the order of about 19,600 m^3/year (8,600 and 11,600 m^3/year for Phase 1 and Phase II respectively). To compensate the infiltration deficit, Low Impact Development (LID) features such as roof downspout disconnection were recommended and three LID infiltration galleries have been designed by D.G. Biddle and Associates (stormwater engineers) to address the infiltration deficit to maintain pre-development infiltration.
- The wastewater assessment indicated that the proposed development of 58 lot estate homes with private, on-site wastewater disposal, would result in a nitrate concentration less than the Ontario Drinking Water Quality Standard of

10 mg/L. The nitrate concentration calculated at the property boundary was 8.14 mg/L.

- Construction excavation dewatering may be required as any proposed sewer and watermain infrastructure will likely intercept the groundwater. Once detailed design for the linear infrastructure and building basements are available, the dewatering assessment is recommended to be completed.
- An unevaluated wetland occupies much of the central and northeast portions of the Site and therefore, there could be some impacts on the local natural features due to the Site development. Set-back distances or buffer zones as prescribed by the KC should be followed to protect the natural features.
- Ongoing long term water level measurements are currently being completed to support the development of a feature-based water balance report.

RESPONSES TO PUBLIC CONCERNS

The following comments were provided to Cambium in response to the Municipality's public council meeting held on Tuesday September 16, 2025. Cambiums follow up response to each comment was included below.

> ***Comment #1*** *- Provide a summary of the hydrogeological assessment report conclusion as well as some additional commentary on the methodology for the "feature-based water balance analysis" and why this is important.*

A summary of the hydrogeological assessment conclusions is provided in the subsection above.

A feature-based water balance is important to evaluate the potential developmental impacts on a wetland feature and to ensure that the detailed stormwater management design will maintain similar levels of surface water runoff to the wetland as well as not significantly disrupt the existing hydroperiod of the wetland. As identified, the Site has a mapped unevaluated wetland (Wetland 1) which occupies much of the central and northeast portions of the Site. The requirement for a feature-based water balance is prompted when a

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proposed development has the potential to alter the wetland catchments water balance either by adding or removing flow to the wetland outside pre-development conditions. Phase I of the proposed development is located entirely outside of Wetland 1 and the 30 m setback so no alterations to flow patterns are expected post-development. Therefore, a feature-based water balance would not be required for Phase 1 the development.

A feature-based water balance is anticipated to be required for Phase II to better understand the hydraulic capacity of and inform site-specific mitigation strategies to the wetland. As identified in the stormwater management plan prepared by D.G Biddle (Biddle), there are anticipated changes to flow patterns expected in post-development with further refinement available during Phase II and the detailed design process (D.G. Biddle & Associates, 2024).

The methodology for a feature-based water balance includes determining the function of the wetland as being a groundwater recharge or a groundwater discharge feature. This is currently being assessed at the Site. Cambium has installed three well nests in Wetland 1 denoted DP1, DP2 and DP3 (see Borehole Location Plan attached). The well nests consisted of a shallow and a deep groundwater well, which groundwater levels are currently being measured from monthly. Using this data, the direction of vertical groundwater flow (vertical gradients) in the wetland will be determined. At the conclusion of one year of monitoring, the wetland will be determined as a recharge or discharge feature. As mentioned previously, the intent of the year long study of the water levels within the wetland is to determine the seasonal hydroperiod of the wetland. Following this, the model assesses the variability in water levels monthly and seasonally to calculate the range of sensitivity and significance of the hydroperiod. Once the model is created for the wetland, it can be assessed and compared to the post-development water balance, including any stormwater discharge, to determine if any impacts may occur or possible methods for mitigation.

> ***Comment #2 -*** *There were concerns about well water quantity at the meeting. It is understood that some wells surrounding the Site are dry right*

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> *now and people are worried that the development will exacerbate the situation for them.*

The development is proposed to be serviced for water and wastewater on-site. In Ontario, the Ministry of Environment Conservation and Parks (MECP) *Procedure D-5-5 Technical Guideline for Private Wells: Water Supply Assessment* (MECP, 1996) provides technical guidance to professionals is land development in assessment of groundwater quality and quantity. Jp2g completed a water supply assessment per Procedure D-5-5 in the *Hydrogeological Investigation and Terrain Analysis* report. The Jp2g report was reviewed and referenced for information to answer this concern.

With respect to water quantity, Procedure D-5-5 states that each future domestic well must provide sufficient water for normal domestic purposes as determined from running pumping tests in test wells. There were four test wells that were advanced into the underlying limestone/dolomite bedrock aquifer at the Site by Jp2g. The depths of the wells ranged from 30 to 55 metres below ground surface. Two of the supply wells were advanced on the west side of the wetland, and two wells were advanced on the east side of Wetland 1.

As per the Jp2g report, hydraulic pumping tests were conducted on each of these four test wells at a pumping rate of 13.6 litres per minute (L/min) for 6 hours. The rate of 13.6 L/min is the minimum pumping rate for a single detached dwelling as per Procedure D-5-5. When one test well was being pumped, the others were used as monitoring wells to monitor water level interference from the pumping well (Jp2g, 2021).

Minimal drawdowns were recorded for the test wells ranging between 0.13 and 1.78 m, with Jp2g noting that the test wells did not report well interference effects on the non pumping wells while pumping the respective test wells. The available drawdown in the pumping wells ranged between 16.61 and 30.25 m at the end of each test indicating ample water remaining in each well. Each test well recovered to 95% of pre-pumping static water levels in the test wells within 5 to 120 minutes indicating a robust aquifer (Jp2g, 2021).

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Additionally, Jp2g calculated the limestone bedrock transmissivities (aquifer's capacity to transmit water) for three of the test wells. The calculated transmissivity was considered high at 44.1 to 142.9 m^2/day. Considering the high transmissivity, the fast recovery rates, and low to minor interference effects, Jp2g concludes that an adequate supply of groundwater from the bedrock aquifer is available for Phase 1 of the Site (Jp2g, 2021).

It is Cambium's opinion that based strictly on the pumping tests data and water supply assessment presented for Phase 1, the work of Jp2g adheres to Procedure D-5-5 and the water supply can be adequately sourced without significant off-site well interference.

Cambium completed supplemental hydraulic tests to validate the Jp2g water supply assessment results at the Site, considering the drought conditions in 2025 and that the previous well testing occurred during spring conditions where groundwater levels are typically higher. Cambium returned to Site on November 4 and 5, 2025, to hydraulically test two of the existing four wells on-site to confirm the conclusions outlined by Jp2g.

One well was tested on each side of the wetland. On the west side of the wetland, the existing well located on proposed Lot 9 (Well ID A104942) was selected to test and denoted as TW1. Test well TW1 had the greatest drawdown during the Jp2g testing, indicating that this well was the lowest yielding well at that time. The existing supply well located on proposed Lot 5 (A104945) was used as a monitoring well and denoted as MW1.

On the east side of the wetland, the existing supply well located on proposed Lot 20 (A104944) was selected to test and denoted as TW2 during the hydraulic pumping test. The existing supply well located on proposed Lot 13 (A104943) was denoted as MW2. Both TW2 (Lot 20) and MW2 (Lot 13) were found to be strong wells during the Jp2g well testing, with drawdowns of 0.37 and 0.93 m after 6 hours of pumping, respectively.

Note, the Client later installed a fifth test well TW3 (Well ID A415246) on the north (Phase II) side of the Site to meet the test well requirement of Procedure D-5-5 for the Phase II submission, as the increase in development area with the

addition of the Phase II lands required one additional well to comply with the Procedure D-5-5 requirements. The location of the test and monitoring wells are presented on Figure 1.

Static water level, stickup, water found elevation (fracture depth) and well depth information for each well is provided in Table 1.

Table 1 On-Site Test Well Information

Well ID	Well Tag Number	Well Type	Well Diameter (m)	Well Stickup (mags) (1)	Well Depth (mbgs) (2)	Ground Surface Elevation (masl) (3)	Static Water Level (mbgs) (4)	Static Water Level (masl)	Water Found Elevation (masl)
TW1	A104942	Drilled	0.15	0.60	43	294.93	13.41	281.52	252.26
TW2	A104944	Drilled	0.15	0.84	30	290.34	8.04	282.30	262.60
MW1	A104945	Drilled	0.15	0.55	30	294.29	14.15	280.14	266.86
MW2	A104943	Drilled	0.15	0.91	55	287.64	11.04	276.60	232.78
TW3	A415246	Drilled	0.15	0.80	73	296.14	13.89	282.25	262.31 / 237.62 / 228.17

(1) mbgs – metres below ground surface

(2) mags – metres above ground surface

(3) Lidar elevation data referenced for approximate ground surface elevations in metres above seal level (masl)

(4) Static water levels measured from test and monitoring wells measured during November 2025 pumping tests.

The pumping test for TW1 was conducted on November 4, 2025. The static water level prior to testing was 13.41 mbgs. The test commenced at 9:30 and ran for a continuous 3 hours. A constant rate of 20 L/min was maintained for the duration of the test, with the exception of the flow rate being accidentally lowered to 14 L/min at 10:15 but was adjusted back to 20 L/min by 10:20. The pump was shut off at 12:40. A drawdown of 4.54 m was observed at the end of the 3 hours. The available drawdown at the end of the test was 23.12 m. The test well recovered to 95% of pre-pumping static levels within 4 hours of the pump shut off. Water levels were monitored in MW1 (closest well) throughout the hydraulic testing on TW1. There was no measurable response in MW1. A hydrograph presenting the pumping test has been attached as Figure 2.

The TW2 pumping test was conducted on November 5, 2025. The static water level prior to testing was 8.04 mbgs. The test commenced at 7:30 and ran for a continuous 3 hours. A constant rate of 20 L/min was initially set for the pumping rate before the flow rate was lowered to 14 L/min due to a substantial increase in drawdown. The flow rate was further reduced to 8 L/min at the 35-minute mark for the same reason. The flow rate was increased to 10 L/min at the 87-minute

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mark to test the sustainable rate of the supply well. The rate was reduced back to 8 L/min from the 127-minute mark and maintained to the end of the test. The pump was shut off at 10:30.

A maximum drawdown of 20.67 m during the test was observed before the flow rate was lowered to 8 L/min for the first time. After the flow rate was increased to 10 L/min and then was reduced again to 8 L/min, recovery was observed during the test which continued into the recovery period after the pump was shut off. TW2 recovered to 42% of pre-pumping static levels within 1 hour of the pump shut off. Water levels were monitored in MW2 throughout the hydraulic testing on TW2. There was no measurable response in MW2 as a result of the pumping. A hydrograph presenting the pumping test has been attached as Figure 3.

The results of the testing completed at wells TW1 and TW2 confirm the previous testing results for these wells (note – TW1 was the lowest yielding of the four tested well in the Jp2g study, while TW2 was the second strongest) and the conclusions of the Jp2g report. The results indicate that these wells can provide the minimum yield, on a daily sustainable basis, as outlined by Procedure D-5-5. The hydraulic testing at TW1 indicated that the well had no issues with supplying water at a rate of 20 L/min, with 95% recovery within 4 hours of the test ending, without inducing a measurable response in the closest monitoring well MW1. The hydraulic testing at TW2 by Cambium indicated that the well was able to sustainably supply water at a rate of 8 L/min. While 8 L/min is the steady state pumping rate the well can produce in perpetuity, and this is below the 13.7 L/min requirement of Procedure D-5-5, this well will be able to support a higher pumping rate for shorter pumping intervals, which is more typical of water usage in a residence. It is noted that regular well use is not continuous, and recovery would occur between drawdown events. If required, storage can be added for this well to accommodate peak demand usage.

The lower sustainable pumping rate for TW2 of 8 L/min, differed from the Jp2g pumping test results from June 2017 where it was pumped at a rate 13.6 L/min during spring conditions, while the confirmatory pumping test was conducted during a historically dry autumn season. It is interpreted that because well TW2

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intercepts water from a shallower fracture (27.7 mbgs, 262.60 masl) compared to the fracture depth for well TW1 (42.7 mbgs, 252.26 masl), that they are different fracture systems. The well record for TW2 (and MW1) indicates that the limestone is porous, however water was only found near the base of the well, at a depth of 27.7 mbgs. Furthermore, the higher initial static water level of 8.04 mbgs (282.30 masl) was likely the result of water stored locally in shallow fractures (i.e. perched) as the limestone is noted as porous; these are interpreted to not be extensive across the Site, so when they were initially dewatered, the pumping rate resulted in the pumping level to decrease significantly and unsustainably, leading to the required decrease in the pumping rate. The pumping rate of 8 L/min was sustained by the deepest fracture in this well, which is shallower than the other tested wells on the Site. The results from TW2 suggest that the water supply fracture is lower yielding than the deeper fractures in the other test wells, resulting in TW2 having a decreased well yield more so during seasonally dry periods, when the shallow fractures where water is in storage, are dewatered, resulting in a steeper drawdown cone compared to well TW1. During spring conditions when groundwater levels are higher, the well yield in TW2 would be much greater, which would explain the higher calculated transitivity in the Jp2g report.

The water levels in TW2 recovered to the static level of the water bearing fracture after the test, which was at an elevation of about 270.3 masl (20.0 mbgs), as the water level in the well was no longer influenced by the shallow perched fractures which were dewatered during the pumping test. It is anticipated that during seasonally wetter periods, TW2 would provide a much higher well yield when the shallow fractures could sustain more water. Regardless, well TW2 would be adequate for domestic usage with a storage system to mitigate peak water use during seasonal dry periods.

Additionally, the Client installed one new test well on the north (Phase II) side of the Site to meet the test well requirement of Procedure D-5-5, for the Phase II submission. The new well was denoted as TW3. The result of this investigation is

attached as an addendum report (Cambium, 2025) and should be read in conjunction with this report.

The hydraulic test for well TW3 discharged a total volume of 9,540 L, which far exceeds the estimated 2,740 L water demand required for a 4 bedroom house including an ARU. Since the well can provide much higher yield on a daily basis than the minimum yield outlined by Procedure D-5-5 and there were no significant influences from water withdrawal at TW3, the results indicate that well TW3 is suitable for water supply for the proposed residential dwelling including the ARU. It should be noted that TW3 is connected to three fractures indicated by the water found depths on the well record, at elevations provided in Table 1. While the elevation of the upper fracture at 262.31 masl is similar to TW2 (water found at 262.60 masl), TW3 is also connected to deeper fractures at 237.62 and 228.17 masl. As such, it is recommended that the future wells at the Site target the deeper fracture system that the on-site wells TW1, MW2 and TW3 are installed into (at elevations from 252 to 228 masl) in order to adequately meet peak usage during seasonally dry periods.

Over the 3 hours of pumping TW1 and TW2, approximately 3,570 L and 1,770 L, respectively, was discharged from the wells. TW1 had ample available drawdown of 23.13 m at the end of the test, and TW2 had 3.7 m of available drawdown at the end of the test. Subsequently, TW3 discharged a volume of 9,540 L over 6 hours of pumping at a pumping rate of 26.5 L/min, and a with drawdown of 5.9 m and a remaining available drawdown of 51.41 m. As all these pumping tests were undertaken during seasonal drought conditions, they would reflect a worst-case scenario of drawdown from water usage.

Overall, all the tested wells except TW2 can support the higher pumping demand of a residence with an ARU of 2,740 L and storage can be added to well TW2 to accommodate peak demand usage. As stated above, future wells at the Site should target the deeper fracture system that the on-site wells TW1, MW2 and TW3 are installed into (at elevations from 252 to 228 masl) to adequately meet peak usage during seasonally dry periods.

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For the Phase II submission, Cambium recommends that a private water well survey is completed to document public water supply in the area of the Site.

> ***Comment #3*** *-Can a description of fracking for additional well-waters be provided? How does the fracking impact surrounding wells?*

Water well fracking or hydrofracking is process where high-pressure water is surged into bedrock fractures to enlarge them and to flush debris from the fractures. This procedure is often done to increase water production in low-yielding wells.

It should be noted that all four of the test wells on-site had adequate well yield and would not require hydrofracking. The well records from the test wells on-site had recommended pumping rates ranging between 13.6 and 37.9 L/min.

A review of the MECP Water Well Information System (WWIS) indicated a total of 73 water supply records within 500 m of the Site (MECP, 2025). A summary report of the MECP well records are attached. Of the 73 supply well records, 71 records were installed into the limestone bedrock aquifer and only 2 of them were installed into overburden. Of all the well records 89% reported a recommended pumping rate above the 13.6 L/min (or 3 imperial gal/min) minimum yield rate for a private water well of Procedure D-5-5. Cambium notes there are eight well records (11% of the supply wells) that have a recommended pumping rate of under 13.6 L/min (5 and 9 L/min). This indicates that the bedrock aquifer yield is not uniform throughout the region. However, even at these reduced pumping rates, a well could meet the daily water demand for a typical four-bedroom dwelling with between 4.2 and 7.5 hours of operation without requiring well hydrofracking. As such, based on the hydrogeological conditions tested on the Site, hydrofracking is not anticipated for any wells drilled on the property.

> ***Comment #4*** *- At the south outlet of the main wetland on Lot 19, the owner is concerned and would like confirmation that no additional water will flow onto his property as a result of the development. The owner says the "runoff" is on his lot now and he is worried more will occur if the wetland is taking flows from the development.*

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Surface water inputs and discharge to the wetland, as well as down-gradient flows, are assessed by D.G. Biddle and Associates (stormwater engineers).

> ***Comment #5 -*** *Can you please provide a justification for the septic calculations by lot? The concern is that 1000 litres/day has been used and that is not sufficient for the size of house that will be built as well as potential ARUs that we are rezoning for.*

The daily sewage flow volume of 1,000 L/day used in nitrate attenuation assessment, is the average daily sewage effluent flow volume outlined in MECP Procedure D-5-4 (MECP, 1996) which was followed in the preparing the calculations. Conversely, a rate of 2,000 L/day is used for a 4-bedroom house under the Ontario Building Code (OBC) to quantify the peak daily sewage flow rate for adequate sizing in the design of a septic system. Septic systems at the Site will be required to be designed based on the maximum daily sewage flows referenced in Part 8 of the OBC. For hydrogeological assessments for rural subdivision developments, the average sewage flow is representative of the typical daily flow for the subdivision lots to assess the nitrate contribution to the receiving aquifer system.

Should the Site include additional residence units (ARU) as a part of the detailed design, the daily sewage flow rates will need to be re-examined. Cambium completed preliminary calculations to confirm feasibility of ARUs. Cambium should be informed of the number of ARUs proposed, and the nitrate assessment should be updated at that time.

Cambium revised the nitrate attenuation calculations to include the Wetland 1 area, from the continuing monitoring at Site, because it is known that shallow groundwater flows through the Wetland 1, and thus can be considered as dilution area for the assessment of nitrate. The available dilution area was determined to be 476,000 m^2 which is the difference between the Site area (485,400 m^2) and the stormwater ponds area for the east and west ponds (10,420 m^2).

A sewage flow of 1,490 L/day was estimated based on the following:

- 1,000 L/day for the proposed single detached dwelling (Procedure D-5-4)

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- 490 L/day for the additional residential unit (calculated using the flow equivalent to 1 bedroom apartment i.e. 1.4 persons per unit at 350 L/p/d = 490 L/day per ARU)

Note that Procedure D-5-4 assumes average usage while septic systems are designed based on maximum peak usage under the OBC.

The predictive nitrate assessment is summarized below with the water balance and nitrate attenuation calculations enclosed.

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Predictive Assessment

Total nitrogen (all species) ultimately converts to nitrate through the wastewater treatment process. Nitrate is considered to be the critical contaminant in sewage effluent. A nitrate loading of 40 mg/L is used to determine the domestic effluent loading from conventional septic systems on the receiving subsurface. To determine the concentration of nitrate at the property boundary, a mass balance calculation was completed as described below:

$$C = \frac{Q_e * C_e + Q_i * C_i}{Q_t}$$

Where:

Q_e – Volume of Wastewater

C_e – Estimated concentration of nitrate in effluent

Q_i – Volume of Available dilution water

C_i – Concentration of nitrate in dilution water

Q_t – Total flow at the Site (Q_e + Q_i)

C – Predicted concentration of nitrate at the property boundary

Table 2 summarizes the predicted nitrate concentration at the property boundary for conventional treatment for both 58 residential units as well as 58 residential units each with an ARU:

Table 2 Nitrate Dilution Calculation (D-5-4)

Parameter	58 Residential Dwellings	58 Residential Dwellings + 58 ARUs
Q_e (L)	58,000	86,420
C_e (mg/L)	40	40
Q_i (L)	289,393	289,393
C_i (mg/L)	0.1	0.1
Q_t (L)	347,393	375,813
C (mg/L)	**6.76**	**9.28**

The nitrate concentration at the property boundary of the Site area is estimated at 6.76 mg/L using conventional wastewater treatment for 58 single detached residential dwellings or 9.28 mg/L for 58 single detached residential dwellings each with an ARU. Therefore, the Site could be developed to accommodate the

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proposed 58 lots including up to 58 ARUs without exceeding a nitrate concentration limit of 10 mg/L at the property boundary.

Predictive Assessment – Eastern Side Only

Cambium prepared an additional assessment of nitrate attenuation of the development area on the east side of the wetland area, based on concerns raised by the peer reviewers. The east side of the development has been determined to be the down-gradient portion of the development, and this portion would not benefit from having the wetland area as a groundwater buffer zone from any potential nitrate plumes as may occur on the west side of the Site. As such, the peer reviewers requested that a nitrate dilution model be provided for the eastern portion of the Site to determine if this area could comply with the requirements of Procedure D-5-4.

The available dilution area on the east side of the wetland was determined to be 124,905 m^2 which is the difference between the eastern Site development area (190,445 m^2) and the east stormwater pond area (5,500 m^2). The area for the western side of the development, the western stormwater pond and the wetland were not included in the available dilution area.

A sewage flow of 1,490 L/day was estimated based on the following:

- 1,000 L/day for the proposed single detached dwelling (Procedure D-5-4)
- 490 L/day for the additional residential unit (calculated using the flow equivalent to 1 bedroom apartment i.e. 1.4 persons per unit at 350 L/p/d = 490 L/day per ARU)

Note that Procedure D-5-4 assumes average usage (1,000 L/day) while septic systems are designed based on maximum peak usage under the OBC.

The predictive nitrate assessment is summarized below with the water balance and nitrate attenuation calculations enclosed.

Total nitrogen (all species) ultimately converts to nitrate through the wastewater treatment process. Nitrate is considered to be the critical contaminant in sewage effluent. A nitrate loading of 40 mg/L is used to determine the domestic effluent

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loading from conventional septic systems on the receiving subsurface. To determine the concentration of nitrate at the property boundary, a mass balance calculation was completed as described below:

$$C = \frac{Q_e * C_e + Q_i * C_i}{Q_t}$$

Where:

Q_e – Volume of Wastewater

C_e – Estimated concentration of nitrate in effluent

Q_i – Volume of Available dilution water

C_i – Concentration of nitrate in dilution water

Q_t – Total flow at the Site (Q_e + Q_i)

C – Predicted concentration of nitrate at the property boundary

Table 2 summarizes the predicted nitrate concentration at the property boundary for conventional treatment for both 26 residential units (no ARUs), and for 26 residential units with 22 ARUs:

Table 3 Nitrate Dilution Calculation (D-5-4)

Parameter	26 Residential Dwellings (no ARUs)	26 Residential Dwellings + 22 ARUs
Q_e (L)	26,000	36,780
C_e (mg/L)	40	40
Q_i (L)	112,658	112,658
C_i (mg/L)	0.1	0.1
Q_t (L)	138,658	149,438
C (mg/L)	**7.58**	**9.92**

The nitrate concentration at the property boundary of the eastern portion of the Site area is estimated at 7.58 mg/L using conventional wastewater treatment for 26 single detached residential dwellings. The nitrate concentration at the property boundary is estimated at 9.92 mg/L for 26 single detached residential dwellings with 22 of the units including an ARU. Should the Client wish to proceed with 26 ARUs, advanced treatment for nitrate reduction can be added for the 4 lots that cannot support having an ARU.

Therefore, the Site could be developed to accommodate the proposed 26 lots including up to 22 ARUs without exceeding a nitrate concentration limit of 10 mg/L at the property boundary utilizing conventional septic systems. If Site plans include 26 ARUs, additional treatment options should be considered.

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Closing

We trust that the information in this submission meets your current requirements. If you have any questions regarding the contents of this report, please contact the undersigned.

Respectfully submitted,

Cambium Inc.

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Encl. *Cambium Qualifications & Limitations*
Borehole Location Plan
MECP Well Records
Water Balance and Nitrate Attenuation Calculations
Figure 1 Site Plan
Figure 2 TW1 Pump Test – November 4, 2025
Figure 3 TW2 Pump Test – November 5, 2025

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Nicole Mountain
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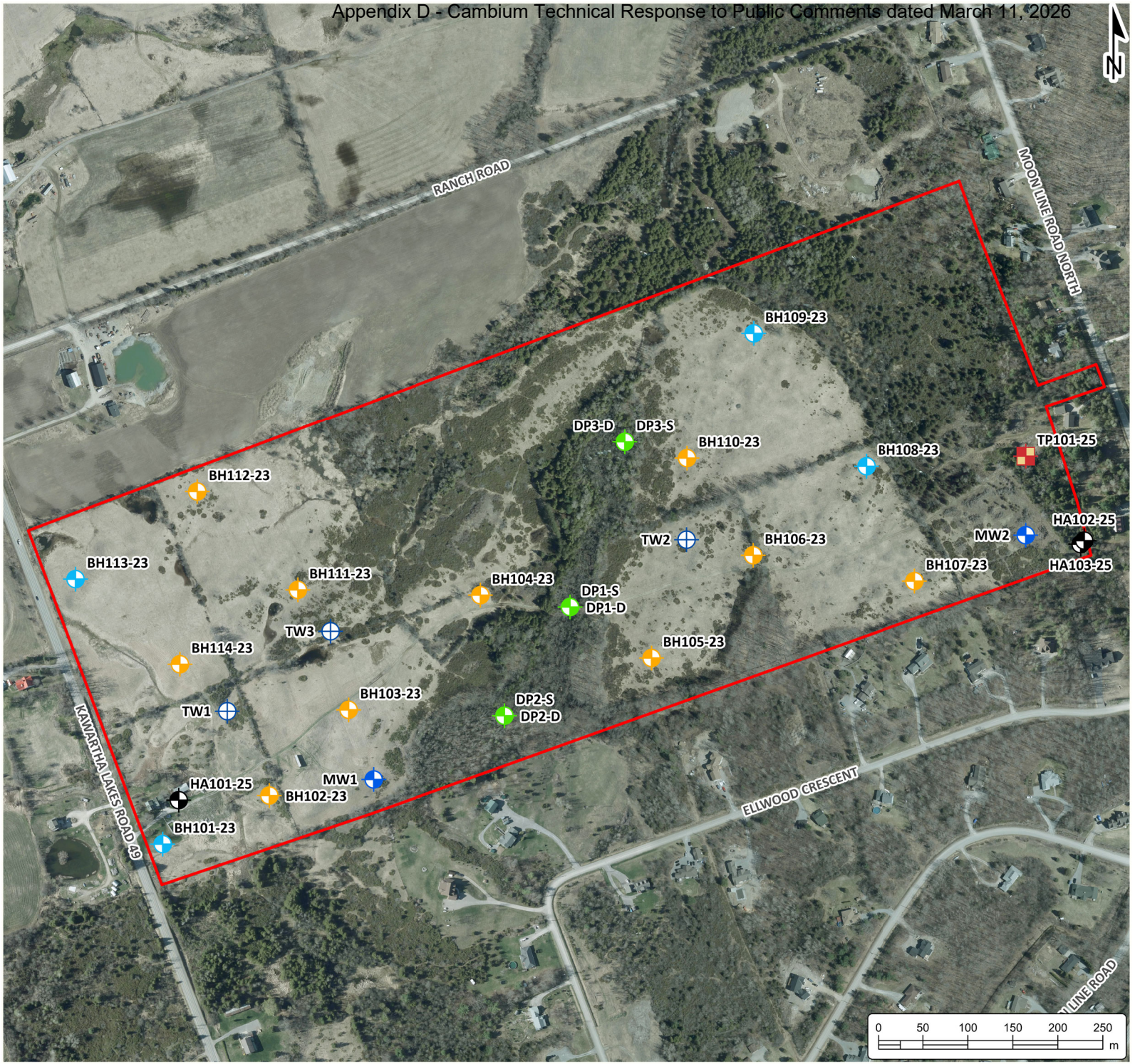

HYDROGEOLOGICAL ASSESSMENT

JEFFERY HOMES
Pt. Lot 19, Conc. 19 - 168 County Road 49
Bobcaygeon, Ontario

LEGEND

- Borehole
- Monitoring Well
- Piezometer
- Hand Auger
- Test Pit
- Monitoring Well (Supply Well)
- Test Well
- Site (approximate)

Notes:
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- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.
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BOREHOLE LOCATION PLAN

Project No.:	17986-003	Date:	January 2026
		Rev.:	3
Scale:	1:6,000	Projection:	NAD 1983 UTM Zone 17N
Created by:	MAT	Checked by:	KW
Figure:	3		

O:\GIS\Projects\17900-17999\17986-003 Jeffery Homes - WWW - Pt. Lot 19 Conc. 19\2025-04-25 Groundwater Configuration 17986-003.aprx

Water Well Records Summary Repo

Produced by Cambium Inc. using MOECP Water Well Information System (WWIS)

All units in meters unless otherwise specified

Well ID: 5101895 **Easting** 695815 **UTM Zone** 17
Construction Date: 11/8/1960 **Northing:** 4937218 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	12.2	**Water Kind:**	FRESH	**Pump Rate (LPM):**	5
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	5
Water First Found:	8.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	3.66				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.05
2	LIMESTONE	3.05	12.19

Well ID: 5101896 **Easting** 694894 **UTM Zone** 17
Construction Date: 1/7/1963 **Northing:** 4936742 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	18.3	**Water Kind:**		**Pump Rate (LPM):**	
Well Diameter (cm):	15.24	**Final Status:**	Abandoned-Su	**Recommended Pump Rate:**	
Water First Found:		**Primary Water Use:**		**Pumping Duration (h:m)**	
Static Level:					

Layer:	Driller's Description	Top:	Bottom:
1	PREVIOUSLY DUG	0.00	6.10
2	CLAY	6.10	10.97
3	LIMESTONE	10.97	18.29

Well ID: 5101897 **Easting** 695837 **UTM Zone** 17
Construction Date: 1/7/1963 **Northing:** 4937169 **Positional Accurac** unknown UTM

Well Depth:	6.4	**Water Kind:**	FRESH	**Pump Rate (LPM):**	27
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	27
Water First Found:	6.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	3.66				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.91
2	LIMESTONE	0.91	6.40

Well ID: 5101898 **Easting** 694880 **UTM Zone** 17
Construction Date: 9/12/1967 **Northing:** 4936687 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	12.8	**Water Kind:**	FRESH	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	11.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	3.66				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	4.27
2	LIMESTONE	4.27	12.80

Well ID: 5105293 **Easting** 695965 **UTM Zone** 17
Construction Date: 1/19/1971 **Northing:** 4937194 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	9.1	**Water Kind:**	FRESH	**Pump Rate (LPM):**	18
Well Diameter (cm):		**Final Status:**	Water Supply	**Recommended Pump Rate:**	18
Water First Found:	4.6	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	3.05				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	3.05
	GRAVEL		
2	LIMESTONE	3.05	9.14
	LIMESTONE		

Well ID: 5105833 **Easting** 694965 **UTM Zone** 17
Construction Date: 1/24/1972 **Northing:** 4936724 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	27.1	**Water Kind:**	SULPHUR	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	19.8	**Primary Water Use:**	Public	**Pumping Duration (h:m)**	8 : 0
Static Level:	9.75				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	2.74
2	LIMESTONE	2.74	26.82
3	LIMESTONE	26.82	27.13

Well ID: 5105939 **Easting** 695665 **UTM Zone** 17
Construction Date: 6/5/1972 **Northing:** 4936874 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	7.9	**Water Kind:**	FRESH	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	5.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	3.05				

Layer:	Driller's Description	Top:	Bottom:
1	PREVIOUSLY DUG	0.00	1.52
2	STONES	1.52	3.35
3	LIMESTONE	3.35	7.92

Well ID: 5106169 **Easting** 695865 **UTM Zone** 17
Construction Date: 12/14/197 **Northing:** 4937274 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	9.1	**Water Kind:**	FRESH	**Pump Rate (LPM):**	18
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	9.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	4 : 20
Static Level:	4.27				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.61
2	CLAY	0.61	4.57
3	LIMESTONE	4.57	9.14

Well ID: 5106171 **Easting** 695715 **UTM Zone** 17
Construction Date: 12/12/197 **Northing:** 4936784 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	12.2	**Water Kind:**	FRESH	**Pump Rate (LPM):**	9
Well Diameter (cm):		**Final Status:**	Water Supply	**Recommended Pump Rate:**	9
Water First Found:	5.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	0.00				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.05
	CLAY		
	CLAY		
	CLAY		
2	LIMESTONE	3.05	12.19
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		

Well ID: 5107786 **Easting** 694815 **UTM Zone** 17
Construction Date: 1/12/1976 **Northing:** 4936874 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	11.6	**Water Kind:**	FRESH	**Pump Rate (LPM):**	9
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	9
Water First Found:	6.4	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	4.57				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	5.79
2	LIMESTONE	5.79	11.58

Well ID: 5107994 **Easting** 695815 **UTM Zone** 17
Construction Date: 6/15/1976 **Northing:** 4936924 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	32.6	**Water Kind:**	FRESH	**Pump Rate (LPM):**	9
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	9
Water First Found:	30.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	9.14				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	8.53
2	LIMESTONE	8.53	18.29
3	LIMESTONE	18.29	28.04
4	LIMESTONE	28.04	32.61

Well ID: 5109218 **Easting** 695915 **UTM Zone** 17
Construction Date: 12/5/1978 **Northing:** 4937324 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth:	7.6	**Water Kind:**	FRESH	**Pump Rate (LPM):**	9
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	5
Water First Found:	7.0	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	3 : 0
Static Level:	1.83				

Layer:	Driller's Description	Top:	Bottom:
1	BOULDERS	0.00	2.13
	BOULDERS		
2	LIMESTONE	2.13	7.62

2	LIMESTONE	2.13	7.62

Well ID: 5110210 **Easting** 694614 **UTM Zone** 17
Construction Date: 4/6/1981 **Northing:** 4937523 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth: 10.1 **Water Kind:** FRESH **Pump Rate (LPM):** 14
Well Diameter (cm): **Final Status:** Water Supply **Recommended Pump Rate:** 14
Water First Found: 7.6 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 2 : 30
Static Level: 4.57

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.61
	TOPSOIL		
2	CLAY	0.61	4.57
	CLAY		
3	STONES	4.57	10.06
	STONES		

Well ID: 5112330 **Easting** 695335 **UTM Zone** 17
Construction Date: 5/21/1987 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 7.0 **Water Kind:** FRESH **Pump Rate (LPM):** 23
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 18
Water First Found: 7.0 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 3.05

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	5.18
2	LIMESTONE	5.18	7.01

Well ID: 5112900 **Easting** 695335 **UTM Zone** 17
Construction Date: 1/13/1988 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 7.3 **Water Kind:** FRESH **Pump Rate (LPM):** 18
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 18
Water First Found: 7.3 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 2 : 30
Static Level: 2.44

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	4.88
2	LIMESTONE	4.88	5.49
3	LIMESTONE	5.49	7.32

Well ID: 5113091 **Easting** 695335 **UTM Zone** 17
Construction Date: 5/26/1988 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 14.3 **Water Kind:** FRESH **Pump Rate (LPM):** 91
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 45
Water First Found: 11.0 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 8.84

Layer:	Driller's Description	Top:	Bottom:
1	UNKNOWN TYPE	0.00	6.10
2	LIMESTONE	6.10	9.14
3	SHALE	9.14	13.11
4	LIMESTONE	13.11	14.33

Well ID: 5113148 **Easting** 695335 **UTM Zone** 17
Construction Date: 6/3/1988 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	10.4	**Water Kind:**	FRESH	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	7.6	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	2.44				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	1.83
2	GRAVEL	1.83	4.88
3	LIMESTONE	4.88	10.36

Well ID: 5113944 **Easting** 695119 **UTM Zone** 17
Construction Date: 7/4/1989 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth:	20.4	**Water Kind:**	SULPHUR	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	14.3	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	7.32				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	0.61
	GRAVEL		
2	CLAY	0.61	2.13
	CLAY		
3	LIMESTONE	2.13	2.74
	LIMESTONE		
4	LIMESTONE	2.74	14.02
	LIMESTONE		
5	LIMESTONE	14.02	14.33
	LIMESTONE		
6	LIMESTONE	14.33	20.42
	LIMESTONE		

Well ID: 5114002 **Easting** 695119 **UTM Zone** 17
Construction Date: 8/4/1989 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth:	24.4	**Water Kind:**	SULPHUR	**Pump Rate (LPM):**	91
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	22.6	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 30
Static Level:	17.98				

Layer:	Driller's Description	Top:	Bottom:
1	SAND	0.00	0.61
2	TOPSOIL	0.61	0.91
3	GRAVEL	0.91	3.66
4	SHALE	3.66	5.79
5	LIMESTONE	5.79	8.23
6	LIMESTONE	8.23	8.53
7	LIMESTONE	8.53	24.38

Well ID: 5114003 **Easting** 695335 **UTM Zone** 17
Construction Date: 8/22/1989 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	6.4	**Water Kind:**	FRESH	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	36
Water First Found:	5.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	1.22				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.05
2	GRAVEL	3.05	5.49
3	LIMESTONE	5.49	6.40

Well ID: 5114237 **Easting** 695335 **UTM Zone** 17
Construction Date: 11/8/1989 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	73.2	**Water Kind:**	FRESH	**Pump Rate (LPM):**	32
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	27
Water First Found:	70.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	18.29				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.91
2	LIMESTONE	0.91	25.91
3	GRANITE	25.91	73.15

Well ID: 5114552 **Easting** 695335 **UTM Zone** 17
Construction Date: 5/16/1990 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	18.3	**Water Kind:**	SULPHUR	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	17.7	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	4.27				

Layer:	Driller's Description	Top:	Bottom:
1		0.00	11.58
2	LIMESTONE	11.58	18.29

Well ID: 5114726 **Easting** 695119 **UTM Zone** 17
Construction Date: 7/13/1990 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth:	15.2	**Water Kind:**	FRESH	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	8.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	6.71				

Layer:	Driller's Description	Top:	Bottom:
1	FINE SAND	0.00	2.13
	FINE SAND		
2	SHALE	2.13	6.40
	SHALE		
3	LIMESTONE	6.40	15.24
	LIMESTONE		

Well ID: 5115565 **Easting:** 695335 **UTM Zone** 17
Construction Date: 11/13/199 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	18.3	**Water Kind:**	Not stated	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	16.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	7.01				

Layer:	Driller's Description	Top:	Bottom:
1	SAND	0.00	1.83
2	LIMESTONE	1.83	3.66
3	LIMESTONE	3.66	18.29

Well ID: 5115634 **Easting** 695119 **UTM Zone** 17
Construction Date: 1/10/1992 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth:	16.8	**Water Kind:**	SULPHUR	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	
Water First Found:	11.0	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	5.49				

Layer:	Driller's Description	Top:	Bottom:
1	FILL	0.00	1.52
2	TOPSOIL	1.52	1.83
3	CLAY	1.83	3.35
4	LIMESTONE	3.35	16.76

Well ID: 5116236 **Easting** 695335 **UTM Zone** 17
Construction Date: 6/21/1993 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	14.3	**Water Kind:**	Not stated	**Pump Rate (LPM):**	50
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	32
Water First Found:	12.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	8.23				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	0.61
2	GRAVEL	0.61	3.96
3	GRAVEL	3.96	5.18
4	SHALE	5.18	6.10
5	LIMESTONE	6.10	12.80
6	SHALE	12.80	13.11
7	LIMESTONE	13.11	14.33

Well ID: 5116668 **Easting** 695335 **UTM Zone** 17
Construction Date: 9/16/1994 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	25.0	**Water Kind:**	FRESH	**Pump Rate (LPM):**	18
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	19.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	7.01				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	4.57
2	LIMESTONE	4.57	19.81
3	LIMESTONE	19.81	22.86

4	LIMESTONE	22.86	24.99

Well ID: 5117354 **Easting** 695119 **UTM Zone** 17
Construction Date: 12/10/199 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth: 13.7 **Water Kind:** SULPHUR **Pump Rate (LPM):** 36
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 27
Water First Found: 11.6 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 2.44

Layer:	Driller's Description	Top:	Bottom:
1	ROCK	0.00	2.13
	ROCK		
	ROCK		
	ROCK		
2	LIMESTONE	2.13	4.27
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
3	LIMESTONE	4.27	13.72
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		

Well ID: 5117358 **Easting** 695335 **UTM Zone** 17
Construction Date: 12/10/199 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 16.8 **Water Kind:** FRESH **Pump Rate (LPM):** 55
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 45
Water First Found: 10.4 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 4 : 0
Static Level: 9.14

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	CLAY	0.30	3.35
3	SHALE	3.35	4.88
4	SHALE	4.88	6.71
5	LIMESTONE	6.71	16.76

Well ID: 5117511 **Easting** 695335 **UTM Zone** 17
Construction Date: 7/3/1997 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 12.2 **Water Kind:** FRESH **Pump Rate (LPM):** 18
Well Diameter (cm): **Final Status:** Water Supply **Recommended Pump Rate:** 18
Water First Found: 10.7 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 2 : 0
Static Level: 3.96

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
	TOPSOIL		
2	CLAY	0.30	5.49
	CLAY		
3	LIMESTONE	5.49	12.19

3	LIMESTONE	5.49	12.19

Well ID: 5117512 **Easting** 695335 **UTM Zone** 17
Construction Date: 7/3/1997 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 28.0 **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** Abandoned-Ot **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** Domestic **Pumping Duration (h:m)**
Static Level:

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	CLAY	0.30	4.57
3	LIMESTONE	4.57	28.04

Well ID: 5117609 **Easting** 695335 **UTM Zone** 17
Construction Date: 10/6/1997 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 16.8 **Water Kind:** SULPHUR **Pump Rate (LPM):** 36
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 36
Water First Found: 15.5 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 3.05

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	2.44
2	CLAY	2.44	6.40
3	LIMESTONE	6.40	16.76

Well ID: 5117612 **Easting** 695119 **UTM Zone** 17
Construction Date: 10/8/1997 **Northing:** 4937591 **Positional Accurac** unknown UTM

Well Depth: 13.7 **Water Kind:** Not stated **Pump Rate (LPM):**
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 23
Water First Found: 9.8 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 3.05

Layer:	Driller's Description	Top:	Bottom:
1	ROCK	0.00	2.74
2	LIMESTONE	2.74	13.72

Well ID: 5118021 **Easting** 695334 **UTM Zone** 17
Construction Date: 3/23/1999 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 30.5 **Water Kind:** **Pump Rate (LPM):** 68
Well Diameter (cm): **Final Status:** Water Supply **Recommended Pump Rate:** 14
Water First Found: **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 : 0
Static Level: 9.14

Layer:	Driller's Description	Top:	Bottom:
1	LIMESTONE	0.00	30.48

Well ID: 5118022 **Easting** 695334 **UTM Zone** 17
Construction Date: 3/22/1999 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth: 33.5 **Water Kind:** FRESH **Pump Rate (LPM):** 14
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 14
Water First Found: 31.7 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 13.72

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	6.10

	1	CLAY	0.00	6.10
	2	LIMESTONE	6.10	7.92
		LIMESTONE		
	3	LIMESTONE	7.92	33.53
		LIMESTONE		

Well ID: 5119092 **Easting** 695615 **UTM Zone** 17
Construction Date: 6/12/2002 **Northing:** 4936928 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	12.2	**Water Kind:**	Not stated	**Pump Rate (LPM):**	32
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	7.0	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 30
Static Level:	4.57				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.35
2	LIMESTONE	3.35	5.49
3	GRAVEL	5.49	6.71
4	LIMESTONE	6.71	12.19

Well ID: 5119305 **Easting** 695658 **UTM Zone** 17
Construction Date: 2/19/2003 **Northing:** 4937771 **Positional Accurac** margin of error : 300 m - 1 km

Well Depth:	15.2	**Water Kind:**	Not stated	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	12.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 30
Static Level:	6.40				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	1.83
2	LIMESTONE	1.83	15.24

Well ID: 5119650 **Easting** 695332 **UTM Zone** 17
Construction Date: 12/2/2003 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	50.3	**Water Kind:**	Not stated	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	45.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 30
Static Level:	8.53				

Layer:	Driller's Description	Top:	Bottom:
1	FILL	0.00	1.22
	FILL		
2	CLAY	1.22	6.40
	CLAY		
3	LIMESTONE	6.40	45.11
	LIMESTONE		
4	SHALE	45.11	50.29
	SHALE		

Well ID: 5119651 **Easting** 695332 **UTM Zone** 17
Construction Date: 12/2/2003 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	21.3	**Water Kind:**	Not stated	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	16.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 30
Static Level:	11.58				

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	6.71
2	LIMESTONE	6.71	16.76
3	SHALE	16.76	17.07
4	LIMESTONE	17.07	21.34

Well ID: 5119652 **Easting** 695116 **UTM Zone** 17
Construction Date: 12/2/2003 **Northing:** 4937590 **Positional Accurac** unknown UTM

Well Depth:	11.0	**Water Kind:**	Not stated	**Pump Rate (LPM):**	45
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	6.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	2 : 0
Static Level:	2.44				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	GRAVEL	0.30	2.74
3	LIMESTONE	2.74	4.88
4	SHALE	4.88	7.62
5	LIMESTONE	7.62	10.97

Well ID: 5119662 **Easting** 695332 **UTM Zone** 17
Construction Date: 12/2/2003 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	22.9	**Water Kind:**	Not stated	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	18
Water First Found:	19.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	10.36				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	4.27
2	LIMESTONE	4.27	22.86

Well ID: 5119681 **Easting** 695332 **UTM Zone** 17
Construction Date: 12/2/2003 **Northing:** 4937012 **Positional Accurac** unknown UTM

Well Depth:	22.9	**Water Kind:**	Not stated	**Pump Rate (LPM):**	36
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	19.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	9.75				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
	TOPSOIL		
2	STONES	0.30	2.44
	STONES		
3	CLAY	2.44	2.74
	CLAY		

4	CLAY	2.74	5.18
	CLAY		
5	LIMESTONE	5.18	7.01
	LIMESTONE		
6	LIMESTONE	7.01	22.86
	LIMESTONE		

Well ID: 5119895 **Easting** 695665 **UTM Zone** 17
Construction Date: 6/29/2004 **Northing:** 4937695 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth: 9.8 **Water Kind:** Not stated **Pump Rate (LPM):** 45
Well Diameter (cm): **Final Status:** Water Supply **Recommended Pump Rate:** 36
Water First Found: 7.6 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 1.52

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	2.13
	CLAY		
2	LIMESTONE	2.13	3.05
	LIMESTONE		
3	LIMESTONE	3.05	4.88
	LIMESTONE		
4	SHALE	4.88	7.62
	SHALE		
5	LIMESTONE	7.62	9.75
	LIMESTONE		

Well ID: 5119896 **Easting** 695690 **UTM Zone** 17
Construction Date: 6/29/2004 **Northing:** 4937709 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** Abandoned-Ot **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** Not Used **Pumping Duration (h:m)**
Static Level:

Layer:	Driller's Description	Top:	Bottom:

Well ID: 5120223 **Easting** 695077 **UTM Zone** 17
Construction Date: 1/28/2005 **Northing:** 4936993 **Positional Accurac** unknown UTM

Well Depth: 26.5 **Water Kind:** Not stated **Pump Rate (LPM):** 36
Well Diameter (cm): **Final Status:** Water Supply **Recommended Pump Rate:** 32
Water First Found: 21.3 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 6.10

Layer:	Driller's Description	Top:	Bottom:
1	PREV. DRILLED	0.00	15.24
2	LIMESTONE	15.24	26.52

Well ID: 5120462 **Easting:** 695169 **UTM Zone** 17
Construction Date: 11/21/200 **Northing:** 4936941 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	27.4	**Water Kind:**		**Pump Rate (LPM):**	32
Well Diameter (cm):		**Final Status:**	Water Supply	**Recommended Pump Rate:**	27
Water First Found:	21.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	12.19				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	6.10
	CLAY		
2	LIMESTONE	6.10	21.95
	LIMESTONE		
3	LIMESTONE	21.95	22.56
	LIMESTONE		
4	LIMESTONE	22.56	27.43
	LIMESTONE		

Well ID: 7049433 **Easting** 695898 **UTM Zone** 17
Construction Date: 9/19/2007 **Northing:** 4937313 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	17.1	**Water Kind:**		**Pump Rate (LPM):**	55
Well Diameter (cm):		**Final Status:**	Water Supply	**Recommended Pump Rate:**	36
Water First Found:	11.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	5 :
Static Level:	2.87				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
	TOPSOIL		
2	CLAY	0.30	4.27
	CLAY		
3	LIMESTONE	4.27	6.71
	LIMESTONE		
4	LIMESTONE	6.71	17.07
	LIMESTONE		

Well ID: 7053759 **Easting** 695669 **UTM Zone** 17
Construction Date: 12/13/200 **Northing:** 4937846 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	18.3	**Water Kind:**	FRESH	**Pump Rate (LPM):**	36
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	36
Water First Found:	13.1	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	11.28				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	6.10
2	CLAY	6.10	7.01
3	LIMESTONE	7.01	18.29

Well ID: 7053804 **Easting** 694993 **UTM Zone** 17
Construction Date: 12/13/200 **Northing:** 4937074 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	24.4	**Water Kind:**	FRESH	**Pump Rate (LPM):**	55
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	19.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	14.08				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	CLAY	0.30	6.10
3	LIMESTONE	6.10	24.38

Well ID: 7116313 **Easting** 695665 **UTM Zone** 17
Construction Date: 12/15/200 **Northing:** 4937413 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	122.0	**Water Kind:**	FRESH	**Pump Rate (LPM):**	251
Well Diameter (cm):	15.80	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:		**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	8.47				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	1.52
	TOPSOIL		
2	LIMESTONE	1.52	42.70
	LIMESTONE		
3	GRANITE	42.70	54.00
	GRANITE		
4	GRANITE	54.00	122.00
	GRANITE		

Well ID: 7126215 **Easting** 695629 **UTM Zone** 17
Construction Date: 7/28/2009 **Northing:** 4937883 **Positional Accurac** margin of error : 10 - 30 m

Well Depth:	18.3	**Water Kind:**	FRESH	**Pump Rate (LPM):**	36
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	36
Water First Found:	8.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 : 0
Static Level:	5.49				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	2.44
	CLAY		
2	CLAY	2.44	4.57
	CLAY		
3	LIMESTONE	4.57	18.29
	LIMESTONE		

Well ID: 7141464 **Easting** 695659 **UTM Zone** 17
Construction Date: 3/16/2010 **Northing:** 4938005 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	13.7	**Water Kind:**	FRESH	**Pump Rate (LPM):**	41
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	8.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	5.49				

Layer:	Driller's Description	Top:	Bottom:

1	CLAY	0.00	2.44
2	CLAY	2.44	5.18
3	LIMESTONE	5.18	7.01
4	SHALE	7.01	8.23
5	LIMESTONE	8.23	13.72

Well ID: 7152131 **Easting** 695263 **UTM Zone** 17
Construction Date: 9/28/2010 **Northing:** 4937131 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth: 61.0 **Water Kind:** FRESH **Pump Rate (LPM):** 9
Well Diameter (cm): 15.87 **Final Status:** Water Supply **Recommended Pump Rate:** 9
Water First Found: 57.9 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 9.14

Layer:	Driller's Description	Top:	Bottom:
1	STONES	0.00	2.43
2	LIMESTONE	2.43	10.36
3	LIMESTONE	10.36	57.91
4	GRANITE	57.91	60.96

Well ID: 7166812 **Easting** 695819 **UTM Zone** 17
Construction Date: 8/5/2011 **Northing:** 4937692 **Positional Accurac** margin of error : 100 m - 300 m

Well Depth: 18.3 **Water Kind:** **Pump Rate (LPM):** 18
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 18
Water First Found: 7.9 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 4.88

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	2.74
	GRAVEL		
2	SHALE	2.74	4.27
	SHALE		
3	LIMESTONE	4.27	18.29
	LIMESTONE		

Well ID: 7174938 **Easting** 695634 **UTM Zone** 17
Construction Date: 1/13/2012 **Northing:** 4938054 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 18.3 **Water Kind:** Untested **Pump Rate (LPM):** 27
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 23
Water First Found: 8.8 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 5.79

Layer:	Driller's Description	Top:	Bottom:
1	GRAVEL	0.00	2.74
	GRAVEL		
2	LIMESTONE	2.74	18.29

2	LIMESTONE	2.74	18.29

Well ID: 7194496 **Easting** 695125 **UTM Zone** 17
Construction Date: 1/3/2013 **Northing:** 4936890 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 24.4 **Water Kind:** Untested **Pump Rate (LPM):** 36
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 32
Water First Found: 21.3 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 13.11

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.66
	CLAY		
2	SHALE	3.66	6.71
	SHALE		
3	LIMESTONE	6.71	15.24
	LIMESTONE		
4	LIMESTONE	15.24	24.38
	LIMESTONE		

Well ID: 7214594 **Easting** 695370 **UTM Zone** 17
Construction Date: 1/10/2014 **Northing:** 4936803 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 91.4 **Water Kind:** FRESH **Pump Rate (LPM):** 23
Well Diameter (cm): 15.56 **Final Status:** Water Supply **Recommended Pump Rate:** 23
Water First Found: 91.4 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 5.18

Layer:	Driller's Description	Top:	Bottom:
1	FILL	0.00	1.83
2	LIMESTONE	1.83	4.27
3	LIMESTONE	4.27	30.48
4	LIMESTONE	30.48	48.77
5	GRANITE	48.77	91.44

Well ID: 7238855 **Easting** 695706 **UTM Zone** 17
Construction Date: 3/23/2015 **Northing:** 4937806 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 18.3 **Water Kind:** Untested **Pump Rate (LPM):** 45
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 45
Water First Found: 9.8 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 6.89

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.66
	CLAY		
2	CLAY	3.66	8.53
	CLAY		
3	SHALE	8.53	9.75
	SHALE		
4	LIMESTONE	9.75	18.29
	LIMESTONE		

Well ID: 7249066 **Easting** 694579 **UTM Zone** 17
Construction Date: 10/5/2015 **Northing:** 4937152 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	21.3	**Water Kind:**	Untested	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	8.5	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	0.88				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	3.66
	CLAY		
	CLAY		
	CLAY		
	CLAY		
	CLAY		
	CLAY		
	CLAY		
2	LIMESTONE	3.66	21.34
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		

Well ID: 7249387 **Easting** 695504 **UTM Zone** 17
Construction Date: 10/5/2015 **Northing:** 4937048 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	25.3	**Water Kind:**	Untested	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	13.7	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	11.58				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	5.49
	CLAY		
2	CLAY	5.49	7.92
	CLAY		
3	LIMESTONE	7.92	25.30
	LIMESTONE		

Well ID: 7266059 **Easting** 695664 **UTM Zone** 17
Construction Date: 7/7/2016 **Northing:** 4938030 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	18.3	**Water Kind:**	Untested	**Pump Rate (LPM):**	68
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	45
Water First Found:	18.3	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	5.79				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	5.18

1	CLAY	0.00	5.18	
2	CLAY	5.18	7.32	
3	LIMESTONE	7.32	18.29	

Well ID: 7268191 **Easting** 695755 **UTM Zone** 17
Construction Date: 8/3/2016 **Northing:** 4937165 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 16.5 **Water Kind:** Untested **Pump Rate (LPM):** 45
Well Diameter (cm): 15.88 **Final Status:** Water Supply **Recommended Pump Rate:** 45
Water First Found: 6.4 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 4.08

Layer:	Driller's Description	Top:	Bottom:
1	STONES	0.00	2.44
2	LIMESTONE	2.44	5.49
3	LIMESTONE	5.49	6.71
4	LIMESTONE	6.71	16.46

Well ID: 7268209 **Easting** 695291 **UTM Zone** 17
Construction Date: 8/3/2016 **Northing:** 4937208 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 21.6 **Water Kind:** Untested **Pump Rate (LPM):** 55
Well Diameter (cm): 15.24 **Final Status:** Water Supply **Recommended Pump Rate:** 36
Water First Found: 7.6 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 2 :
Static Level: 4.11

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	GRAVEL	0.30	4.27
3	LIMESTONE	4.27	7.62
4	LIMESTONE	7.62	10.97
5	LIMESTONE	10.97	21.64

Well ID: 7280438 **Easting** 695711 **UTM Zone** 17
Construction Date: 2/2/2017 **Northing:** 4937359 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 21.3 **Water Kind:** Untested **Pump Rate (LPM):** 45
Well Diameter (cm): 15.88 **Final Status:** Water Supply **Recommended Pump Rate:** 36
Water First Found: 18.3 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 9.30

Layer:	Driller's Description	Top:	Bottom:

	CLAY	0.00	1.52
	CLAY		
2	CLAY	1.52	3.05
	CLAY		
3	LIMESTONE	3.05	21.34
	LIMESTONE		

Well ID: 7280447 **Easting** 695300 **UTM Zone** 17
Construction Date: 2/2/2017 **Northing:** 4937119 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 46.3 **Water Kind:** Untested **Pump Rate (LPM):** 18
Well Diameter (cm): 15.88 **Final Status:** Water Supply **Recommended Pump Rate:** 18
Water First Found: 42.7 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 10.55

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	2.44
	CLAY		
	CLAY		
	CLAY		
2	LIMESTONE	2.44	46.33
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		

Well ID: 7292009 **Easting** 694747 **UTM Zone** 17
Construction Date: 8/8/2017 **Northing:** 4937289 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 42.7 **Water Kind:** **Pump Rate (LPM):** 5
Well Diameter (cm): 15.56 **Final Status:** Water Supply **Recommended Pump Rate:**
Water First Found: 42.7 **Primary Water Use:** Domestic **Pumping Duration (h:m)** :
Static Level: 9.75

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.91
2	SAND	0.91	2.13
3		2.13	42.67

Well ID: 7292010 **Easting** 695637 **UTM Zone** 17
Construction Date: 8/8/2017 **Northing:** 4937485 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: 54.9 **Water Kind:** FRESH **Pump Rate (LPM):** 16
Well Diameter (cm): 15.56 **Final Status:** Water Supply **Recommended Pump Rate:** 16
Water First Found: 54.9 **Primary Water Use:** Domestic **Pumping Duration (h:m)** 1 :
Static Level: 8.08

Layer:	Driller's Description	Top:	Bottom:
1	SAND	0.00	7.92
2	DOLOMITE	7.92	54.86

Well ID: 7292011 **Easting** 695259 **UTM Zone** 17
Construction Date: 8/8/2017 **Northing:** 4937480 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	30.5	**Water Kind:**	FRESH	**Pump Rate (LPM):**	16
Well Diameter (cm):	15.56	**Final Status:**	Water Supply	**Recommended Pump Rate:**	
Water First Found:	27.7	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	6.64				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	SAND	0.30	5.79
3	LIMESTONE	5.79	30.48

Well ID: 7292012 **Easting** 694910 **UTM Zone** 17
Construction Date: 8/8/2017 **Northing:** 4937213 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	30.5	**Water Kind:**	FRESH	**Pump Rate (LPM):**	16
Well Diameter (cm):	15.56	**Final Status:**	Water Supply	**Recommended Pump Rate:**	16
Water First Found:	27.4	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	10.39				

Layer:	Driller's Description	Top:	Bottom:
1	TOPSOIL	0.00	0.30
2	SAND	0.30	4.88
3	LIMESTONE	4.88	30.48

Well ID: 7299050 **Easting** 695754 **UTM Zone** 17
Construction Date: 11/10/201 **Northing:** 4937457 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	18.3	**Water Kind:**	Untested	**Pump Rate (LPM):**	16
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	16
Water First Found:	16.8	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	8.81				

Layer:	Driller's Description	Top:	Bottom:
1	SAND	0.00	0.91
2	LIMESTONE	0.91	18.29

Well ID: 7317100 **Easting** 695388 **UTM Zone** 17
Construction Date: 8/17/2018 **Northing:** 4937025 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	30.5	**Water Kind:**	FRESH	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	23
Water First Found:	7.6	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	4.27				

Layer:	Driller's Description	Top:	Bottom:
1	BOULDERS	0.00	2.13
	BOULDERS		
2	LIMESTONE	2.13	30.48
	LIMESTONE		

Well ID: 7317101 **Easting** 695385 **UTM Zone** 17
Construction Date: 8/17/2018 **Northing:** 4936956 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	48.8	**Water Kind:**		**Pump Rate (LPM):**	9
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	7
Water First Found:	42.7	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	10.97				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	6.10
	CLAY		
	CLAY		
	CLAY		
2	LIMESTONE	6.10	48.77
	LIMESTONE		
	LIMESTONE		
	LIMESTONE		

Well ID: 7335058 **Easting** 695386 **UTM Zone** 17
Construction Date: 6/13/2019 **Northing:** 4937239 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	18.9	**Water Kind:**	FRESH	**Pump Rate (LPM):**	23
Well Diameter (cm):	15.88	**Final Status:**	Water Supply	**Recommended Pump Rate:**	9
Water First Found:	12.2	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	25 :
Static Level:	5.26				

Layer:	Driller's Description	Top:	Bottom:
1	STONES	0.00	1.22
2	MEDIUM SAND	1.22	4.88
3	LIMESTONE	4.88	18.90

Well ID: 7345764 **Easting** 695156 **UTM Zone** 17
Construction Date: 10/30/201 **Northing:** 4937051 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	61.0	**Water Kind:**	FRESH	**Pump Rate (LPM):**	14
Well Diameter (cm):	15.24	**Final Status:**	Water Supply	**Recommended Pump Rate:**	14
Water First Found:	57.9	**Primary Water Use:**	Domestic	**Pumping Duration (h:m)**	1 :
Static Level:	9.14				

Layer:	Driller's Description	Top:	Bottom:
1	CLAY	0.00	6.10
	CLAY		
2	LIMESTONE	6.10	60.05
	LIMESTONE		
3	GRANITE	60.05	60.96
	GRANITE		

Well ID: 7427375 **Easting** 695277 **UTM Zone** 17
Construction Date: 8/25/2022 **Northing:** 4936854 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:		**Water Kind:**		**Pump Rate (LPM):**	
Well Diameter (cm):		**Final Status:**		**Recommended Pump Rate:**	
Water First Found:		**Primary Water Use:**		**Pumping Duration (h:m)**	
Static Level:					

Layer:	Driller's Description	Top:	Bottom:

Well ID: 7460923 **Easting** 694848 **UTM Zone** 17
Construction Date: 9/27/2023 **Northing:** 4936700 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** **Pumping Duration (h:m)**
Static Level:

Layer: **Driller's Description** **Top:** **Bottom:**

Well ID: 7460924 **Easting** 694844 **UTM Zone** 17
Construction Date: 9/27/2023 **Northing:** 4936902 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** **Pumping Duration (h:m)**
Static Level:

Layer: **Driller's Description** **Top:** **Bottom:**

Well ID: 7460925 **Easting** 694769 **UTM Zone** 17
Construction Date: 9/27/2023 **Northing:** 4937120 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** **Pumping Duration (h:m)**
Static Level:

Layer: **Driller's Description** **Top:** **Bottom:**

Well ID: 7460926 **Easting** 694949 **UTM Zone** 17
Construction Date: 9/27/2023 **Northing:** 4936812 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** **Pumping Duration (h:m)**
Static Level:

Layer: **Driller's Description** **Top:** **Bottom:**

Well ID: 7467455 **Easting** 694825 **UTM Zone** 17
Construction Date: 12/27/202 **Northing:** 4937436 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth: **Water Kind:** **Pump Rate (LPM):**
Well Diameter (cm): **Final Status:** **Recommended Pump Rate:**
Water First Found: **Primary Water Use:** **Pumping Duration (h:m)**
Static Level:

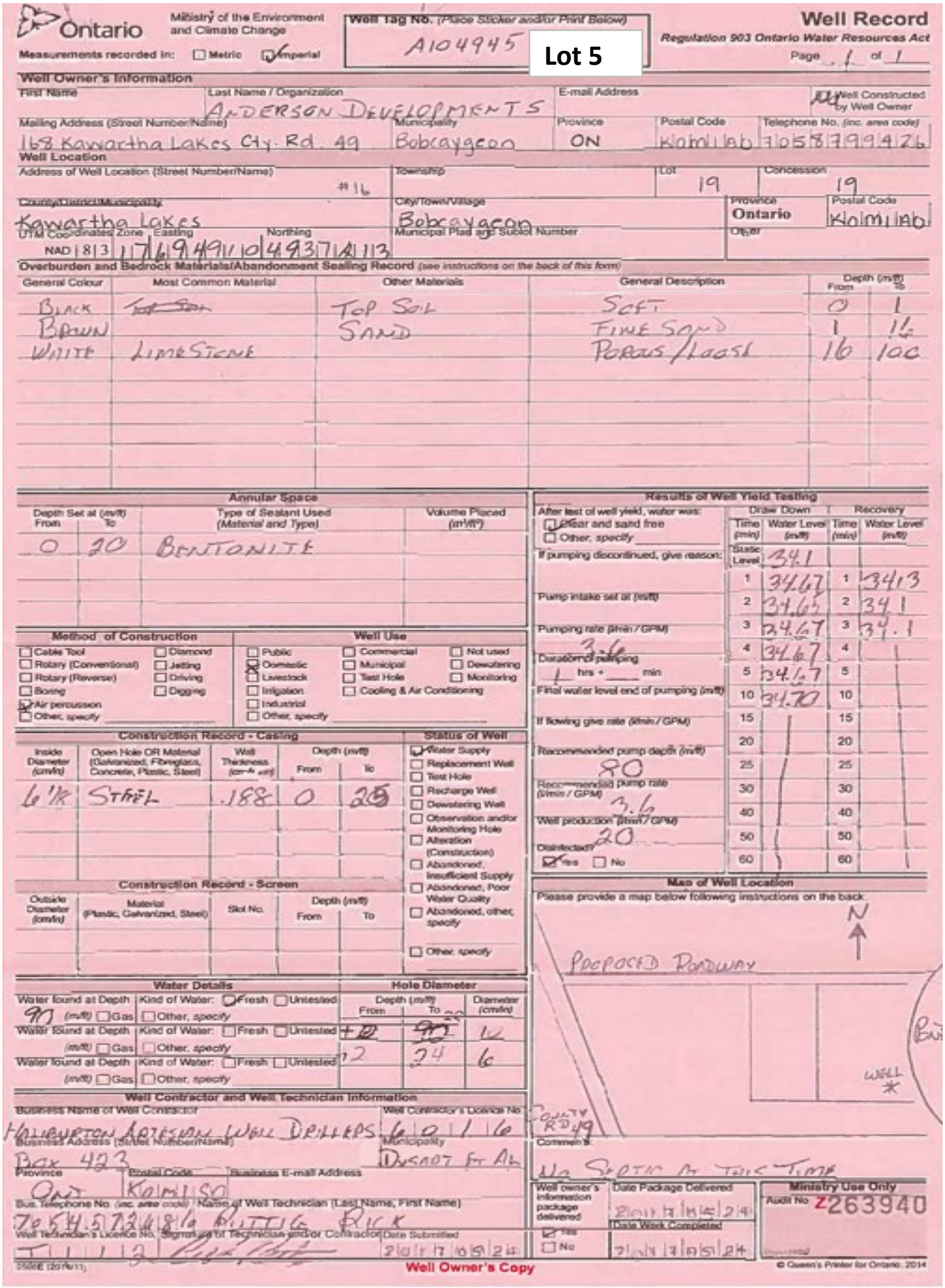
Layer: **Driller's Description** **Top:** **Bottom:**

Well ID: 7481298 **Easting:** 695191 **UTM Zone:** 17
Construction Date: 7/2/2024 **Northing:** 4937240 **Positional Accurac** margin of error : 30 m - 100 m

Well Depth:	**Water Kind:**	**Pump Rate (LPM):**
Well Diameter (cm):	**Final Status:**	**Recommended Pump Rate:**
Water First Found:	**Primary Water Use:**	**Pumping Duration (h:m)**
Static Level:		

Layer:	Driller's Description	Top:	Bottom:

MW1



Ontario
Ministry of the Environment and Climate Change
Measurements recorded in: ☐ Metric ☑ Imperial

Well Tag No. (Place Sticker and/or Print Below) A104945

Lot 5

Well Record
Regulation 903 Ontario Water Resources Act
Page 1 of 1

Well Owner's Information
First Name | Last Name / Organization: ANDERSON DEVELOPMENTS | E-mail Address | ☐ Well Constructed by Well Owner
Mailing Address (Street Number/Name): 168 Kawartha Lakes Cty. Rd. 49 | Municipality: Bobcaygeon | Province: ON | Postal Code: K0M1A0 | Telephone No. (inc. area code): 7058799426

Well Location
Address of Well Location (Street Number/Name): #16 | Township | Lot: 19 | Concession: 19
County/District/Municipality: Kawartha Lakes | City/Town/Village: Bobcaygeon | Province: Ontario | Postal Code: K0M1A0
UTM Coordinates: Zone: NAD 83 | Easting: 17 694911 | Northing: 4937121 13 | Municipal Plan and Sublot Number | Other

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	To
BLACK	~~TOP SOIL~~	TOP SOIL	SOFT	0	1
BROWN		SAND	FINE SAND	1	16
WHITE	LIMESTONE		POROUS / LOOSE	16	100

Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
0	20	BENTONITE	

Method of Construction
☐ Cable Tool ☐ Diamond ☐ Rotary (Conventional) ☐ Jetting ☐ Rotary (Reverse) ☐ Driving ☐ Boring ☐ Digging ☑ Air percussion ☐ Other, specify

Well Use
☐ Public ☐ Commercial ☐ Not used ☑ Domestic ☐ Municipal ☐ Dewatering ☐ Livestock ☐ Test Hole ☐ Monitoring ☐ Irrigation ☐ Cooling & Air Conditioning ☐ Industrial ☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	To
6 1/4	STEEL	.188	0	25

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	To

Status of Well
☑ Water Supply ☐ Replacement Well ☐ Test Hole ☐ Recharge Well ☐ Dewatering Well ☐ Observation and/or Monitoring Hole ☐ Alteration (Construction) ☐ Abandoned, Insufficient Supply ☐ Abandoned, Poor Water Quality ☐ Abandoned, other, specify ☐ Other, specify

Water Details

Water found at Depth (m/ft)	Kind of Water	Hole Diameter Depth From	To	Diameter (cm/in)
97	☐ Gas ☐ Fresh ☐ Untested ☐ Other, specify			
	☐ Gas ☐ Fresh ☐ Untested ☐ Other, specify	12	97	6
	☐ Gas ☐ Fresh ☐ Untested ☐ Other, specify	12	24	6

Results of Well Yield Testing
After test of well yield, water was: ☑ Clear and sand free ☐ Other, specify
If pumping discontinued, give reason:
Pump intake set at (m/ft)
Pumping rate (l/min / GPM)
Duration of pumping: 1 hrs + ___ min
Final water level end of pumping (m/ft)
If flowing give rate (l/min / GPM)
Recommended pump depth (m/ft): 80
Recommended pump rate (l/min / GPM): 3.6
Well production (l/min / GPM): 20
Disinfected? ☑ Yes ☐ No

	Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level		34.1		
1	1	34.67	1	34.13
2	2	34.65	2	34.1
3	3	34.67	3	34.1
4	4	34.67	4	
5	5	34.67	5	
10	10	34.70	10	
15			15	
20			20	
25			25	
30			30	
40			40	
50			50	
60			60	

Map of Well Location
Please provide a map below following instructions on the back.

Comments: NO SEPTIC AT THIS TIME

Well Contractor and Well Technician Information
Business Name of Well Contractor: HALLIBURTON ROTHSON WELL DRILLERS | Well Contractor's Licence No.: 6011
Business Address (Street Number/Name): Box 423 | Municipality: DUNSFORD | Province: ONT | Postal Code: K0M1S0 | Business E-mail Address
Bus. Telephone No. (inc. area code): 7054572818 | Name of Well Technician (Last Name, First Name): KITTIG, RICK
Well Technician's Licence No.: T1112 | Signature of Technician and/or Contractor | Date Submitted: 2017/05/24

Well owner's information package delivered: ☑ Yes ☐ No
Date Package Delivered: 2017/05/24
Date Work Completed: 2017/05/24

Ministry Use Only
Audit No. Z263940

Well Owner's Copy

TW2

Ontario — Ministry of the Environment and Climate Change

Measurements recorded in: ☐ Metric ☑ Imperial

Well Tag No. (Place Sticker and/or Print Below): A104944 — Lot 20

Well Record

Regulation 903 Ontario Water Resources Act

Page 1 of 1

Well Owner's Information

First Name	Last Name / Organization	E-mail Address	☐ Well Constructed by Well Owner
	ANDERSON DEVELOPMENTS		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
168 Kawartha Lakes Cty. Rd. 49	Bobcaygeon	ON	K0M1A0	7052797914216

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
#48		19	19

County/District/Municipality	City/Town/Village	Province	Postal Code
Kawartha Lakes	Bobcaygeon	Ontario	K0M1A0

UTM Coordinates Zone	Easting	Northing	Municipal Plan and Sublot Number	Other
NAD 83 17	691525	4949371	4180	

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
BRN	TOP SOIL		LOOSE	0	1
BRN	SAND		11	1	19
GRY	LIMESTONE		BOULDERS	19	100

Annular Space

Depth Set at (m/ft) From	Depth Set at (m/ft) To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
0	22	BENTONITE	

Method of Construction

☐ Cable Tool ☐ Diamond ☐ Rotary (Conventional) ☐ Jetting ☐ Rotary (Reverse) ☐ Driving ☐ Boring ☐ Digging ☑ Air percussion ☐ Other, specify

Well Use

☐ Public ☐ Commercial ☐ Not used ☑ Domestic ☐ Municipal ☐ Dewatering ☐ Livestock ☐ Test Hole ☐ Monitoring ☐ Irrigation ☐ Cooling & Air Conditioning ☐ Industrial ☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	Depth (m/ft) To
6 1/8	STEEL	.188	0	21

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No	Depth (m/ft) From	Depth (m/ft) To

Status of Well

☑ Water Supply ☐ Replacement Well ☐ Test Hole ☐ Recharge Well ☐ Dewatering Well ☐ Observation and/or Monitoring Hole ☐ Alteration (Construction) ☐ Abandoned, Insufficient Supply ☐ Abandoned, Poor Water Quality ☐ Abandoned, other, specify ☐ Other, specify

Water Details

Water found at Depth	Kind of Water	Hole Diameter Depth From	Hole Diameter Depth To	Hole Diameter (cm/in)
91 (m/ft) ☐ Gas	☑ Fresh ☐ Untested ☐ Other, specify			
(m/ft) ☐ Gas	☐ Fresh ☐ Untested ☐ Other, specify	10	100	10
(m/ft) ☐ Gas	☐ Fresh ☐ Untested ☐ Other, specify	42	20	6

Results of Well Yield Testing

After test of well yield, water was: ☑ Clear and sand free ☐ Other, specify

If pumping discontinued, give reason:

Pump intake set at (m/ft): 80

Pumping rate (l/min / GPM): 3.6

Duration of pumping: 1 hrs + ____ min

Final water level end of pumping (m/ft):

If flowing give rate (l/min / GPM):

Recommended pump depth (m/ft): 80

Recommended pump rate (l/min / GPM):

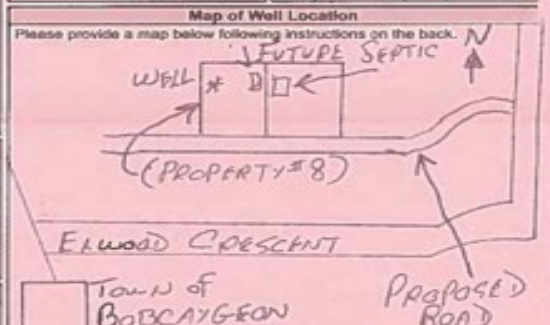
Well production (l/min / GPM): 7

Disinfected? ☑ Yes ☐ No

Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level	21.80		
1	22.9	1	21.85
2	22.55	2	21.73
3	22.45	3	21.67
4	22.35	4	21.67
5	22.35	5	21.66
10	22.45	10	21.65
15	22.47	15	21.64
20	22.50	20	21.64
25	22.60	25	
30		30	
40		40	
50		50	
60		60	

Map of Well Location

Please provide a map below following instructions on the back.



Comments: WELL ONLY NO SEPTIC YET

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
HALIBURTON ARTESIAN WELL DRILLERS	6016

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
Box 423 HALIBURTON	DYSART ET AL	ONT	K0M1S0	

Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)	Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
7054457286	RUTTIG, RICK	7112		2017052216

Well owner's information package delivered: ☑ Yes ☐ No

Date Package Delivered: 2017052216

Date Work Completed: 2017052216

Ministry Use Only — Audit No. Z263939 — Received

05008E (2014/11)

Well Owner's Copy

MW2

Ontario — Ministry of the Environment and Climate Change

Measurements recorded in: ☐ Metric ☑ Imperial

Well Tag No. (Place Sticker and/or Print Below): A104943

Lot 13

Well Record

Regulation 903 Ontario Water Resources Act

Page 1 of 1

Well Owner's Information

First Name	Last Name / Organization	E-mail Address	Well Constructed by Well Owner
	ANDERSON DEVELOPMENTS		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
168 Kawartha Lakes Cty Rd. 49	Bobcaygeon	ON	K0M1A0	7059475946

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
*1		19	19

County/District/Municipality	City/Town/Village	Province	Postal Code
Kawartha Lakes	Bobcaygeon	Ontario	K0M1A0

UTM Coordinates: NAD 83 Zone 17 Easting 696637 Northing 4937485

Municipal Plan and Sublot Number: Other:

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
BROWN	SAND	GRAVEL	LOOSE	0	26
GREY/WHITE	DOLOMITE			26	180

Annular Space

Depth Set at (m/ft) From	Depth Set at (m/ft) To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
0	26	BENTONITE	

Method of Construction

☐ Cable Tool ☐ Diamond ☐ Rotary (Conventional) ☐ Jetting ☐ Rotary (Reverse) ☐ Driving ☐ Boring ☐ Digging ☑ Air percussion ☐ Other, specify

Well Use

☐ Public ☐ Commercial ☐ Not used ☑ Domestic ☐ Municipal ☐ Dewatering ☐ Livestock ☐ Test Hole ☐ Monitoring ☐ Irrigation ☐ Cooling & Air Conditioning ☐ Industrial ☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	Depth (m/ft) To
6 1/8	STEEL	.188	0	160

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	Depth (m/ft) To

Status of Well

☑ Water Supply ☐ Replacement Well ☐ Test Hole ☐ Recharge Well ☐ Dewatering Well ☐ Observation and/or Monitoring Hole ☐ Alteration (Construction) ☐ Abandoned, Insufficient Supply ☐ Abandoned, Poor Water Quality ☐ Abandoned, other, specify ☐ Other, specify

Water Details

Water found at Depth (m/ft)	Kind of Water	Hole Diameter Depth From (m/ft)	Hole Diameter Depth To (m/ft)	Diameter (cm/in)
160 (m/ft) ☐ Gas	☑ Fresh ☐ Untested ☐ Other, specify	0	160	10
(m/ft) ☐ Gas	☐ Fresh ☐ Untested ☐ Other, specify	160	223	6
(m/ft) ☐ Gas	☐ Fresh ☐ Untested ☐ Other, specify			

Results of Well Yield Testing

After test of well yield, water was: ☑ Clear and sand free ☐ Other, specify

If pumping discontinued, give reason:

Pump intake set at (m/ft): 160'

Pumping rate (l/min / GPM): 3.6

Duration of pumping: 1 hrs + ____ min

Final water level end of pumping (m/ft): 29.6'

If flowing give rate (l/min / GPM):

Recommended pump depth (m/ft): 160'

Recommended pump rate (l/min / GPM): 3.6

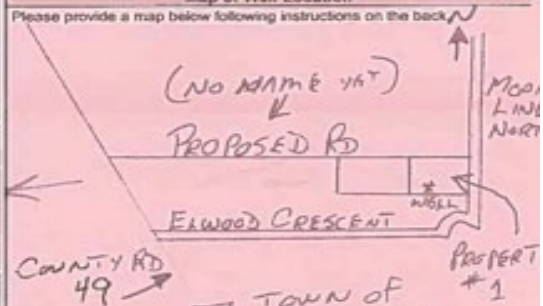
Well production (l/min / GPM): 3.0

Disinfected? ☑ Yes ☐ No

	Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level		26 1/2		
1		30.65	1	30.3
2		31.25	2	29.8
3		31.6	3	29.65
4		31.9	4	29.61
5		32.1	5	29.6
10		32.7	10	
15		32.75	15	
20		32.76	20	
25		32.75	25	
30			30	
40			40	
50			50	
60			60	

Map of Well Location

Please provide a map below following instructions on the back.



Comments: NO SEPTIC SYSTEM YET

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
HALIBURTON ARTESIAN WELL DRILLERS	6016

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
BOX 423 HALIBURTON	DISART ETAL	ONT	K0M1S0	

Bus. Telephone No. (inc. area code): 7054457286

Name of Well Technician (Last Name, First Name): RUTTAN, RICK

Well Technician's Licence No.: 7112

Signature of Technician and/or Contractor:

Date Submitted: 2023/05/23

Well owner's information package delivered: ☐ Yes ☐ No

Date Package Delivered: 2023/05/23

Date Work Completed: 2023/05/23

Ministry Use Only — Audit No. Z263938

05068E (2014/11)

Well Owner's Copy

TW1

Ontario — Ministry of the Environment and Climate Change

Well Record — Regulation 903 Ontario Water Resources Act — Page 1 of 1

Well Tag No. (Place Sticker and/or Print Below): A104948 — **Lot 9**

Measurements recorded in: ☐ Metric ☒ Imperial

Well Owner's Information

First Name	Last Name / Organization	E-mail Address	☐ Well Constructed by Well Owner
	ANDERSON DEVELOPMENTS		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
168 Kawartha Lakes Cty Rd #49	Bobcaygeon	Ont	K0M1A0	7058799426

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
#12		19	19

County/District/Municipality	City/Town/Village	Province	Postal Code
Kawartha Lakes	Bobcaygeon	Ontario	K0M1A0

UTM Coordinates: Zone — Easting — Northing: NAD 83 17 694714 4949372 89

Municipal Plan and Sublot Number: — Other:

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
BROWN	SOIL		SAFT	0	3
BROWN	SAND		"	3	7
WHITE		DOLAMITE	"	7	140

Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
0	20	BENTONITE	

Method of Construction

☐ Cable Tool ☐ Diamond ☒ Rotary (Conventional) ☐ Jetting ☐ Rotary (Reverse) ☐ Driving ☐ Boring ☐ Digging ☒ Air percussion ☐ Other, specify

Well Use

☐ Public ☐ Commercial ☐ Not used ☒ Domestic ☐ Municipal ☐ Dewatering ☐ Livestock ☐ Test Hole ☐ Monitoring ☐ Irrigation ☐ Cooling & Air Conditioning ☐ Industrial ☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	To
6 1/8	STEEL	.188	0	22

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	To

Status of Well

☒ Water Supply ☐ Replacement Well ☐ Test Hole ☐ Recharge Well ☐ Dewatering Well ☐ Observation and/or Monitoring Hole ☐ Alteration (Construction) ☐ Abandoned, Insufficient Supply ☐ Abandoned, Poor Water Quality ☐ Abandoned, other, specify ☐ Other, specify

Water Details

Water found at Depth (m/ft)	Gas	Kind of Water	Other, specify
140	☐	☒ Fresh ☒ Untested	
	☐	☐ Fresh ☐ Untested	
	☐	☐ Fresh ☐ Untested	

Hole Diameter

Depth (m/ft) From	To	Diameter (cm/in)
138	140	6 1/8
+2	22	10
+2	20	6

Results of Well Yield Testing

After test of well yield, water was: ☒ Clear and sand free ☐ Other, specify

If pumping discontinued, give reason:

Pump intake set at (m/ft)

Pumping rate (l/min / GPM): 3.6

Duration of pumping: 1 hrs + ____ min

Final water level end of pumping (m/ft)

If flowing give rate (l/min / GPM)

Recommended pump depth (m/ft): 140'

Recommended pump rate (l/min / GPM)

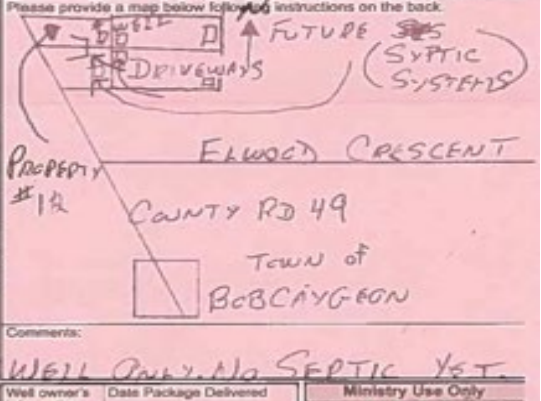
Well production (l/min / GPM): 10

Disinfected? ☒ Yes ☐ No

	Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level		32'		
1	37.55		1	34.80
2	37.65		2	34.00
3	37.50		3	33.40
4	37.55		4	33.20
5	37.45		5	33.10
10	37.26		10	32.88
15	37.17		15	32.76
20	37.40		20	32.76
25	37.30		25	32.66
30	37.40		30	32.64
40	37.40		40	
50	37.32		50	1
60	37.18		60	32

Map of Well Location

Please provide a map below following instructions on the back.



Comments: WELL ONLY. No SEPTIC YET.

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
HALIBURTON ARTESIAN WELL DRILLERS	6016

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
Box 423 Haliburton	Dysart et Al	Ont	K0M1S0	

Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)	Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
7054572686	BUTTIG, RICK	T01112		2017 05 09

Well owner's information package delivered: ☒ Yes ☐ No

Date Package Delivered: 2017 05 19

Date Work Completed: 2017 05 09

Ministry Use Only

Audit No: Z263937

Recorded

0506E (2014/11)

Well Owner's Copy

Ontario — Ministry of the Environment, Conservation and Parks

Well Record
Regulation 903 Ontario Water Resources Act

Well Tag No. (Place Sticker and/or Print Below): A415246
Tag#:A415246

Measurements recorded in: ☐ Metric ☒ Imperial

Page 1 of 2

Well Owner's Information

First Name	Last Name/Organization	E-mail Address	☐ Well Constructed by Well Owner
	Jeffery Homes		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
1200 Airport Blvd, Suite 201	Oshawa	ON		

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
168 County Rd. 49	Trent Lakes (Harvey)	19	19

County/District/Municipality	City/Town/Village	Province	Postal Code
Peterborough	Bobcaygeon	Ontario	K0M 1A0

UTM Coordinates	Zone	Easting	Northing	Municipal Plan and Sublot Number	Other
NAD 8 3	17	6948738	4937391		

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
				0	240
	See Attached				

Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m^3/ft^3)
0	3	Bentonite Chips	0.71
3	20	Bentonite Grout	15

Method of Construction

☐ Cable Tool ☐ Diamond
☐ Rotary (Conventional) ☐ Jetting
☐ Rotary (Reverse) ☐ Driving
☐ Boring ☐ Digging
☒ Air percussion
☒ Other, specify Dual rotary

Well Use

☐ Public ☐ Commercial ☐ Not used
☒ Domestic ☐ Municipal ☐ Dewatering
☐ Livestock ☐ Test Hole ☐ Monitoring
☐ Irrigation ☐ Cooling & Air Conditioning
☐ Industrial
☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	Depth (m/ft) To
6.25	steel	0.188	+3	25.5
6	open hole		25.5	240

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	Depth (m/ft) To

Status of Well

☒ Water Supply
☐ Replacement Well
☐ Test Hole
☐ Recharge Well
☐ Dewatering Well
☐ Observation and/or Monitoring Hole
☐ Alteration (Construction)
☐ Abandoned, Insufficient Supply
☐ Abandoned, Poor Water Quality
☐ Abandoned, other, specify
☐ Other, specify

Water Details

Water found at Depth	Kind of Water	Hole Diameter Depth (m/ft) From	To	Diameter (cm/in)
111 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify	0	20	10
192–198 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify	20	240	6
223–224 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify			

Results of Well Yield Testing

After test of well yield, water was:
☒ Clear and sand free
☐ Other, specify

If pumping discontinued, give reason:

Pump intake set at (m/ft): 230

Pumping rate (l/min / GPM): 7

Duration of pumping: 6 hrs + min

Final water level end of pumping (m/ft): 64.54

If flowing give rate (l/min/GPM):

Recommended pump depth (m/ft): 230

Recommended pump rate (l/min/GPM): 7

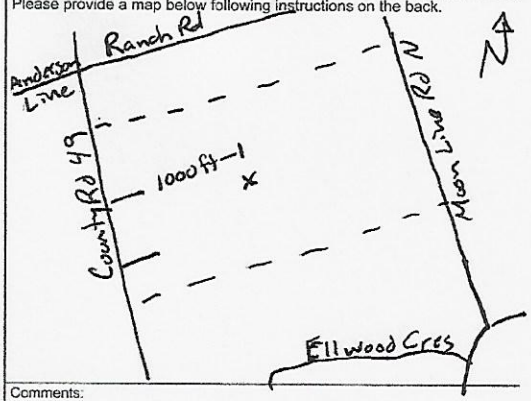
Well production (l/min/GPM): 7

Disinfected? ☒ Yes ☐ No

Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level	45.20		
1	49.15	1	59.00
2	51.27	2	55.55
3	52.86	3	52.90
4	54.15	4	50.98
5	55.18	5	49.66
10	58.40	10	47.40
15	60.07	15	46.94
20	61.10	20	46.67
25	61.68	25	46.50
30	62.12	30	46.38
40	62.78	40	46.24
50	63.10	50	46.17
60	63.34	60	46.15

Map of Well Location

Please provide a map below following instructions on the back.



Comments:

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
G. Hart & Sons Well Drilling Ltd.	2662

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
PO Box 850 Fenelon Falls		Ontario	K0M 1N0	ghart ghart.ca

Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)	Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
705 887 3331	Robinson, Brandon / Watson, Bryan	4802 / 2441		

Well owner's information package delivered	Date Package Delivered	Date Work Completed
☒ Yes ☐ No	mailed	2025 11 13

Ministry Use Only
Audit No. Z437433
Received

G. HART & Son
Well Drilling Ltd.
Box 850, Fenelon Falls, Ont.
K0M 1N0

Tel:(705)-887-3331
Fax:(705)887-4788
E-Mail ghart@ghart.ca

WELL LOG

WELL TAG # :A415246

168 County Road 49, Bobcaygon,ON

General Colour	Most Common Material	Other Materials	General Description	Depth From (Feet)	Depth To (Feet)
Black	Topsoil			0	4
Brown	Silty sand & Gravel			4	21
Grey	Clay & Gravel			21	23
Grey	Broken Limestone			23	24
Grey	Limestone			24	137
White & Black	Limestone			137	164
Grey & Green	Limestone			164	181
Grey & White	Limestone			181	196
Grey & Red	Limestone			196	198
Green	Limestone			198	199
Green & Red	Limestone			199	203.5
Grey	Limestone			203.5	204
Green & Red	Limestone			204	204.5
Red	Limestone			204.5	205
Grey, Red & Green	Limestone			205	206
Brown & Red	Limestone			206	207.5
Red	Limestone			207.5	214
Grey, Pink & Black	Granite			214	240

Appendix D - Cambium Technical Response to Public Comments dated March 11, 2026

Water Balance Calculations

THORNTHWAITE-TYPE MONTHLY WATER-BALANCE MODEL

modified from Dingman 2015: Box 6-8 (pg 299) using ET model of Hamon (1963)

		Input Data				Computed Values							
											Surplus	342	mm/yr
Weather Station Location:	**Peterborough Trent U**					**Latitude:**	**44.2**	**degree**					
Solar Declination (degree)	-20.6	-12.6	-1.5	10.0	19.0	23.1	21.0	13.4	2.6	-9.0	-18.5	-23.0	
DayLength (hr)*	9.1	10.3	11.8	13.3	14.6	15.3	14.9	13.8	12.3	10.8	9.5	8.7	
Available Water Storage Capacity		0.18	m/m		**Root Depth**	1500	mm			**SOILmax**	270.0	mm	

MONTHLY WATER BALANCE DATA

Temperatures in C, water-balance terms in mm.

Month:	J	F	M	A	M	J	J	A	S	O	N	D	Year
TEMPERATURE (T)	-8.4	-6.5	-1.3	6.3	12.8	18.0	20.7	19.4	15.0	8.4	2.4	-4.0	
PRECIPITATION (P)	57.3	48.8	56.5	66.4	88.7	83.0	73.6	87.0	92.4	77.0	85.5	66.0	882
RAIN	22.4	23.1	34.0	60.9	88.7	83.0	73.6	87.0	92.4	75.7	73.3	35.0	749
SNOW	35	26	23	6	0	0	0	0	0	1	12	31	133
MELT FACTOR (F)	0.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.40	0.00	
PACK	73	99	121	0	0	0	0	0	0	0	7	38	
MELT	0	0	0	127	0	0	0	0	0	1	5	0	133
INPUT (W)	22	23	34	188	89	83	74	87	92	77	78	35	882
POTENTIAL ET (PET)	0	0	0	41	70	97	115	98	65	39	22	0	548
NET INPUT (ΔW)	22	23	34	147	19	-14	-41	-11	27	38	56	35	
SOIL MOISTURE (SOIL)	270	270	270	270	270	256	220	211	238	270	270	270	
ΔSOIL	0	0	0	0	0	-14	-36	-9	27	32	0	0	
ET	0	0	0	41	70	97	110	96	65	39	22	0	540
SURPLUS=W-ET-DSOIL	22	23	34	147	19	0	0	0	0	6	56	35	342

Notes:

Precipitation, Rain, Temperature, and Latitude are inputted parameters

SOILmax = available water storage capacity * root depth

m = month

D = Day length (hrs) $=2*\cos^{-1}(-\tan(\text{Latitude})*\tan(\text{Declination}))/0.2618$ [calculation is in radians]

$SNOW_m = P_m - RAIN_m$

$F_m = 0$ if $T_m <= 0^oC$; $F_m = 0.167*T_m$ if $0^oC<T_m<6^oC$; $F_m = 1$ if $T_m>=6^oC$

$PACK_m = (1-F_m)*(SNOW_m+PACK_{m-1})$

$MELT = F_m*(SNOW_m+PACK_{m-1})$

$W_m = RAIN_m+MELT_m$.

PET = 0 if $T_m<0$; otherwise $PET = 2.98*0.611*\exp(17.3*T_m/(T_m+237))/(T_m+237.2)*$Number of days in month [Hamon ET model (1963)]

$\Delta W_m = W_m - PET_m$

$SOIL = \min\{[\Delta W_m+SOIL_{m-1}], SOILmax\}$, if $\Delta Wm>0$; otherwise $SOIL = SOIL_{m-1} * \exp(\Delta W/SOILmax)$

$\Delta SOIL = SOIL_{m-1} - SOIL_m$

ET = PET if W_m > PET; otherwise, $ET=W_m-\Delta SOIL$

Nitrate Attenuation

Calculations for Subdivision Developments

Input Data			Computed Values
Areas	Total		
LOT AREA (m^2)	485400		
BLDG FOOTPRINT (m^2)	0		
SWM POND AREA (m^2)	10420		
Available Infiltration Area (m^2)	474980		

Surplus water		Infiltration Factor	
0.342	m/yr	Rolling	0.2
0.000937	m/day	Silt, sand,gravel till	0.3
445.219966	m^3/day	Woodland/Cultivated	0.15
		Total	0.65

Infiltrated water				
0.00060927	m/day			
289.392978	m^3/day	Runoff	155.8269882	m^3/day

PREDICTED NITRATE CONCENTRATIONS

Combined Concentrations at Property Boundaries

58 Lots	58 Lots + 58 ASUs
58000	86420
40	40
289393	289393
0.1	0.1
347393	375813
6.76	**9.28**

Nitrate Attenuation - Eastern Side of Site Only

Calculations for Subdivision Developments

Input Data			Computed Values
Areas	Total		
LOT AREA (m^2) (East Side only)	190445		
BLDG FOOTPRINT (m^2)	0		
SWM POND AREA (m^2)	5500		
Available Infiltration Area (m^2)	184945		

Surplus water		Infiltration Factor	
0.342	m/yr	Rolling	0.2
0.000937	m/day	Silt, sand,gravel till	0.3
173.357208	m^3/day	Woodland/Cultivated	0.15
		Total	0.65

Infiltrated water				
0.00060927	m/day			
112.682185	m^3/day	Runoff	60.6750228	m^3/day

PREDICTED NITRATE CONCENTRATIONS

Combined Concentrations at Property Boundaries

26 Lots	26 Lots + 22 ASUs
26000	36780
40	40
112682	112682
0.1	0.1
138682	149462
7.58	**9.92**

TECHNICAL RESPONSE TO PUBLIC COMMENTS

JEFFREY HOMES
Pt Lot 19, Conc. 19 – 168 County Road 49
Bobcaygeon, Ontario

LEGEND

- Monitoring Well
- Test Well
- Pumping Test Discharge Direction
- Wetland Boundary (D.G. Biddle & Associates)
- Site (approximate)

Notes:
- This document contains information licensed under the Open Government License - Ontario.
- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.
- Cambium Inc. makes every effort to ensure this map is free from errors but cannot be held responsible for any damages due to error or omissions. This map should not be used for navigation or legal purposes. It is intended for general reference use only.

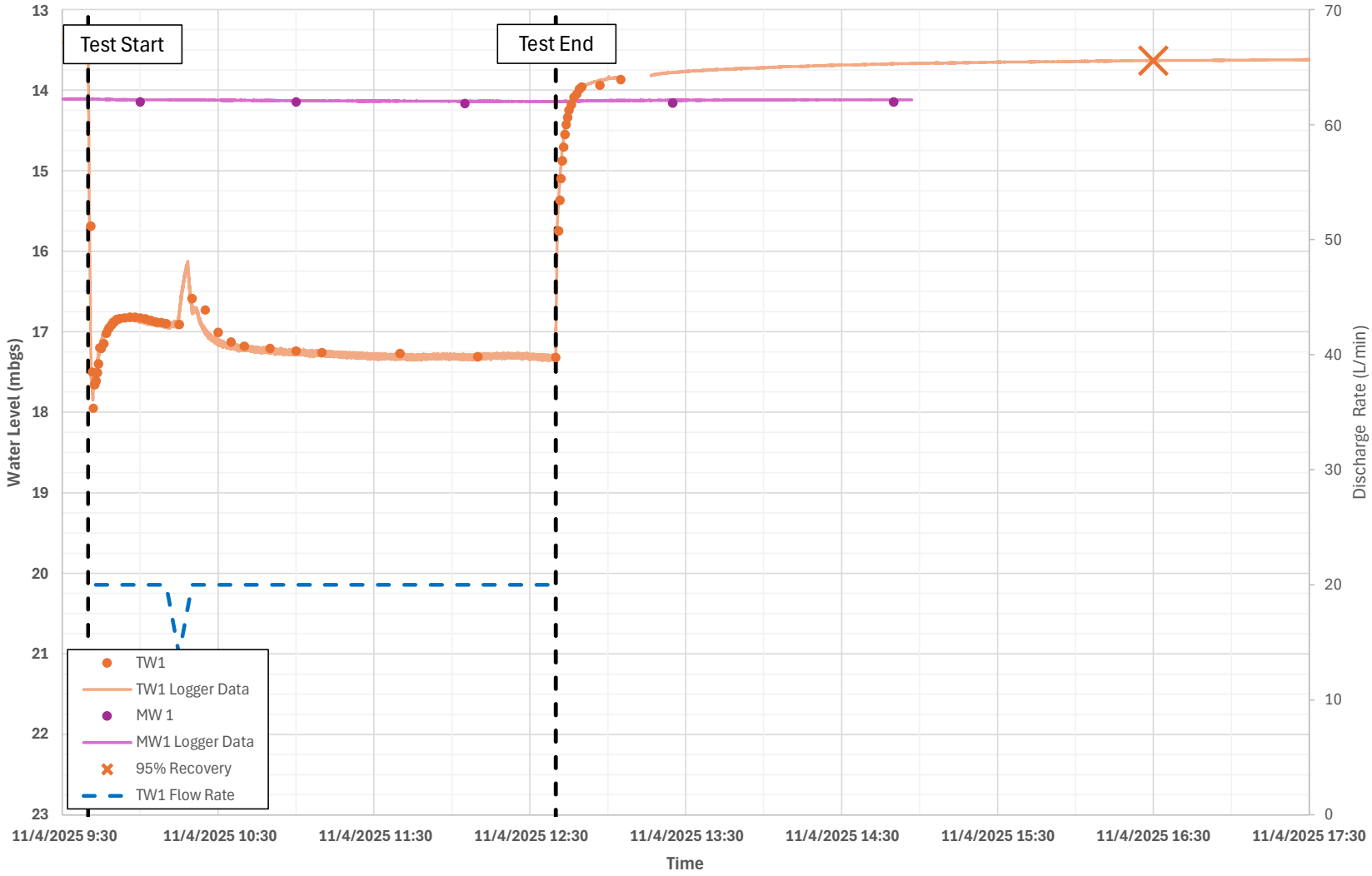
CAMBIUM
194 Sophia Street
Peterborough, Ontario, K9H 1E5
Tel: (705) 742.7900 Fax: (705) 742.7907
www.cambium-inc.com

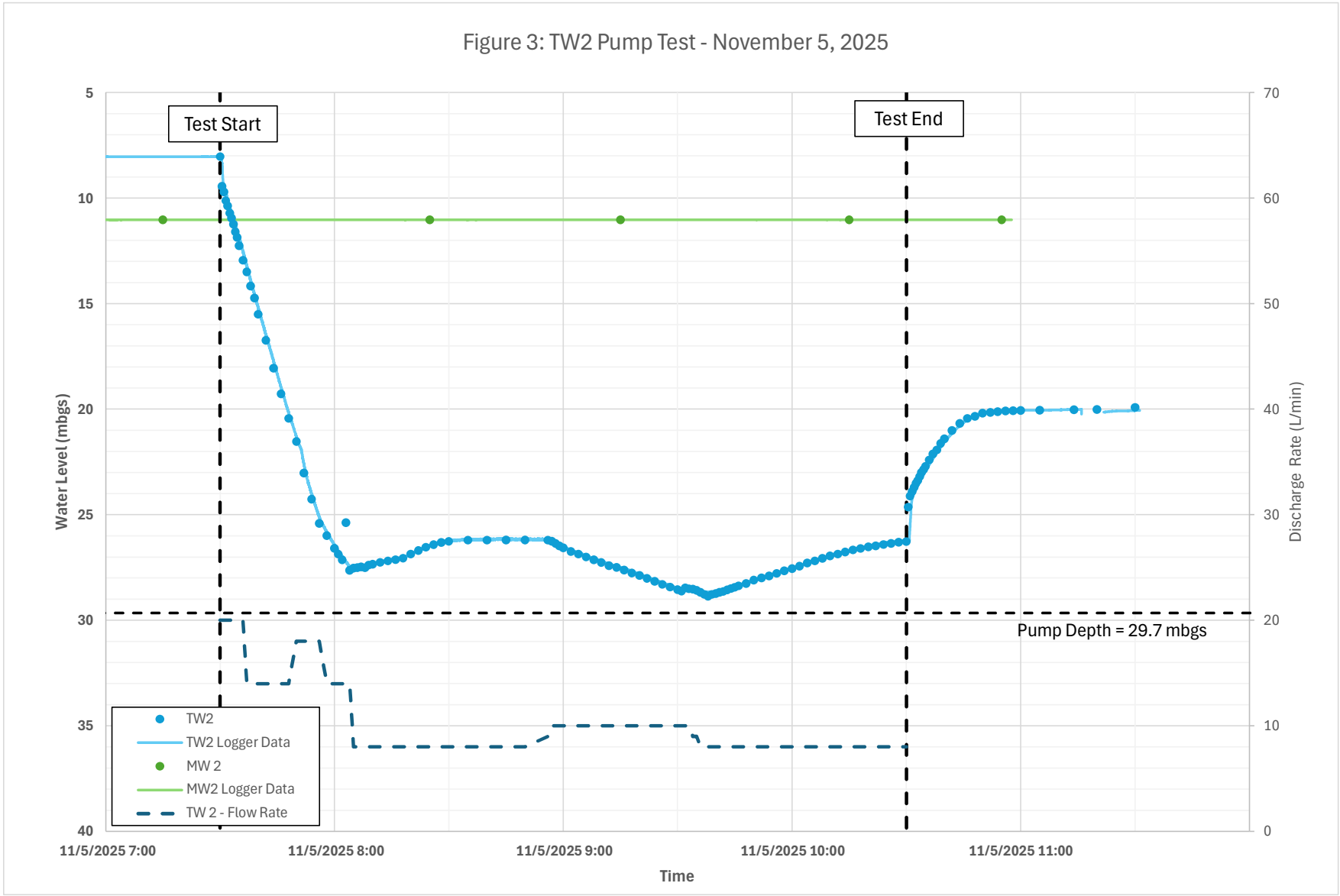
SITE PLAN

Project No.:	Date:
24373-001	November 2025
Scale: 1:6,000	Projection: NAD 1983 UTM Zone 17N
Created by: DBC	Checked by: WY
Figure: 1	

O:\GIS\MXDs\17900-17999\17986-003 Jeffery Homes - WWW - Pt. Lot 19 Conc. 19\2025-11-26 Technical Response.aprx

Figure 2: T W1 Pump Test - November 4, 2025





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March 11, 2026

Jeffery Homes
1200 Airport Blvd, Suite 201
Oshawa, ON
L1J 8P5

Attn: Scott Jeffery

Re: Addendum – Pumping Test for Part of Lot 19, Concession 19, Municipality of Trent Lakes, County of Peterborough
Cambium Reference: 17986-003

Dear Scott Jeffery,

Jeffrey Homes (Client) retained Cambium Inc. (Cambium) to complete additional hydrogeological testing at 168 County Road 49, Part Lot 19, Concession 19, Municipality of Trent Lakes (previously known as Township of Galway-Cavendish, County of Peterborough (Site). It is understood that the Client is pursuing draft plan approval from Municipality of Trent Lakes (Municipality) for two phases of a residential subdivision (herein referred to as Phases I and II).

As part of the Phase II submission, one test well was drilled, installed, and hydraulically tested on the north (Phase II) side of the Site to meet the test well requirement of the Ministry of Environment Conservation and Parks (MECP) *Procedure D-5-5 Technical Guideline for Private Wells: Water Supply Assessment* (MECP, 1996). Cambium has prepared a response to public comments from a public council meeting recently held on Tuesday September 16, 2025. This letter addendum should be read in conjunction with Cambium's *Technical Response to Public Comments* (Cambium, 2026) .

A test well (Well Tag A415246) was installed on the north side of the Site on November 7, 2025, by G. Hart & Sons Well Drilling Ltd. The well record is attached to the letter. The well record identified that the well was installed into 6.4 metres of Silty sand and gravel to clay and gravel overburden before limestone bedrock was encountered. Limestone bedrock extended to a depth of 65.2 mbgs where granite bedrock was encountered. The well was drilled to a final depth of

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73 mbgs and was installed into granite. This well was labelled as TW3 during the hydraulic testing.

ON-SITE WELLS

Previously, four test wells were installed at Site for water supply testing detailed in the report completed by Jp2g (2021). Well TW3 was installed the Phase II lands to meet the test well requirements outlined in Procedure D-5-5, as the size increase with the addition of the Phase II lands required one additional well.

Two test wells were previously installed on the west side of the wetland with one well installed on the proposed lot 9 (A104942) and one installed on proposed lot 5 (A104945). These wells were denoted as TW1 and MW1, respectively, by Cambium during the supplemental hydraulic testing (Cambium, 2026).

Two test wells were previously installed on the east side of the wetland with one well installed on the proposed lot 20 (A104944) and one well installed on the proposed lot 13 (A104943). These test wells were denoted as TW2 and MW2, respectively, by Cambium (2026).

The wells on the west side of the wetland (TW1 and MW1) were monitored during the pumping test on TW3. The wells on the east side of the wetland (TW2 and MW2) were not monitored during the pumping test as they are located further away from the pumping well. Additionally, it is noted that the test and monitoring wells are all installed into the limestone bedrock aquifer, so any potential impacts should be noted at the wells closer to the test well.

Well records are provided as an attachment to this letter. The location of the wells is presented in Figure 1. Static water level, stickup, water found depth and well depth information for each well is provided in Table 1.

March 11, 2026

Table 1 On-Site Test Well Information

Well ID	Well Tag Number	Well Type	Well Diameter (m)	Well Stickup (mags)(1)	Well Depth (mbgs)(2)	Ground Surface Elevation (masl)(3)	Static Water Level (mbgs)(4)	Static Water Level (masl)	Water Found Elevation (masl)
TW1	A104942	Drilled	0.15	0.60	43	294.93	13.41	281.52	252.26
TW2	A104944	Drilled	0.15	0.84	30	290.34	8.04	282.30	262.60
MW1	A104945	Drilled	0.15	0.55	30	294.29	14.15	280.14	266.86
MW2	A104943	Drilled	0.15	0.91	55	287.64	11.04	276.60	232.78
TW3	A415246	Drilled	0.15	0.80	73	296.14	13.89	282.25	262.31 / 237.62 / 228.17

(1) mbgs – metres below ground surface

(2) mags – metres above ground surface

(3) Lidar elevation data referenced for approximate ground surface elevations in metres above seal level (masl)

(4) Static water levels measured from test and monitoring wells measured during November 2025 pumping tests.

TW3 PUMPING TEST METHODOLOGY

A 6-hour pumping test was completed at test well TW3 on November 13, 2025, for a continuous 6-hour period at an average rate of 26.5 L/min. Procedure D-5-5 states that the daily per-person requirement for water withdrawal is 450 L/per person/day. The likely number of persons per well which, for a single family residence, shall be the number of bedrooms plus one. A minimum of four bedrooms shall be used in the calculation, which equates to 5 people per dwelling. In order to determine the water requirements of the additional residence unites (ARU), the flow equivalent to 1 bedroom apartment i.e. 1.4 persons per unit - 350L/p/d for average daily sewage flow = 490 L/d per ARU. In total. The daily water demand per dwelling was calculated to be approximately 2,740 L/day. The continuous 6-hour period at an average pumping rate of 26.5 L/min is a total of 9,540 L/day.

Prior to the pumping test, the well contractor shocked the test well using liquid chlorine bleach to 200 mg/L. Solinst pressure transducer Leveloggers (loggers) were installed in TW3, TW1, and MW1 to monitor water levels before, during, and after the test prior to the pumping test. Barometric pressure was monitored by a separate logger and used for atmospheric pressure compensation. Manual measurements were also recorded during the pumping tests in TW3, TW1, and MW1 to verify automated measurements and mitigate the results in the event of a logger failure.

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March 11, 2026

The well contractor installed a clean, disinfected submersible pump in TW3 prior to testing. Water levels in the test wells were monitored prior to the pumping tests to confirm static conditions before pumping began. The discharge outlet for pumping was placed approximately 15 m downslope direction from the well.

Groundwater quality samples were collected from test well TW3 in the final 30 minutes of the pumping test, when the chlorine was at a concentration less than 0.05 mg/L. The samples were collected from the disinfected outlet hose and handled using nitrile gloves. The samples were put on ice in a sample cooler provided by SGS and were stored at a temperature between 0°C and 10°C.

The samples were submitted to SGS for analysis of the physical, general chemical, and microbiological parameters listed in ODWQS Tables 1, 2, and 3 of Procedure D-5-5. The Certificates of Analysis for the samples are attached.

Field parameters (i.e., turbidity, residual chlorine, pH, conductivity, temperature and Total Dissolved Solids (TDS) were measured regularly from the discharge water during each pumping test.

TW3 PUMPING TEST RESULTS

The static water level in test well TW3 was 13.89 mbgs on November 13, 2025, prior to the commencement of the pumping test. The pump was installed at a depth of approximately 72 mbgs. The available drawdown in the well was therefore approximately 57.31 m (height of static water level above pump).

The pumping test commenced at 9:30 and ran for a continuous duration of 6 hours. A discharge flow rate of 26.5 L/min was set and maintained for the duration of the test. The pump was shut off at 15:30.

Steady-state water level conditions were not achieved, and drawdown was observed throughout the test duration. The maximum drawdown reported at the end of the test was 5.90 m, which equates to a water level of 19.79 mbgs. The remaining available drawdown was approximately 51.41 m (90% of the total available drawdown at the start of the test).

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Professional Engineers Ontario

APGO

March 11, 2026

A total of approximately 9,540 L was pumped from the well during the pumping test. Following pumping cessation, the water level in the well recovered to within 0.3 m of pre-test static water levels (to 14.19 mbgs) by 16:19 on November 13, 2025. This represents 95% water level recovery within 49 minutes, and an available drawdown of 57.01 m (99% of initial available drawdown). A hydrograph presenting the pumping test has been provided as Figure 1.

Field measurements measured for turbidity, chlorine concentration, temperature, conductivity, pH, and TDS during the pumping test are presented below in Table 2.

Table 2 TW3 Field Measurements

Time	Turbidity (NTU)	Chlorine (mg/L)	Temperature °C	Conductivity (µS/cm)	pH	TDS (ppm)
9:50	5.45	> 8.3	8.4	661	7.35	471
10:40	2.04	5.1	9.4	679	7.14	483
11:30	1.33	1.8	9.6	673	7.09	477
12:45	0.63	0.56	9.5	670	7.29	476
13:40	0.58	0.44	8.7	674	7.16	478
14:30	0.8	0.33	7.8	673	7.19	478
15:20	-	0.04	7.6	667	7.36	477

The field parameter measurements indicate that the water quality discharged from well TW3 had generally stabilized upon completion of the pumping test. Further, the turbidity was below 5 NTU, and the residual chlorine was reported as below 0.05 mg/L at the end of the test.

MONITORING WELL INFLUENCES

Water withdrawal at TW3 did not have an observed influence on any of the two on-site wells monitored during the pumping test (TW1 and MW1). A straight-line trend to the manual measurements at the two monitoring wells is provided on Figure 2. The minimal variations in water level observed on the hydrographs are interpreted to be environmental fluctuation in the wells.

CAMBIUM

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Construction Testing & Inspections

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Locations
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Kingston
Barrie
Ottawa
Whitby

Laboratory
Peterborough

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March 11, 2026

WATER QUALITY

Groundwater samples were collected from TW3 were compared against the Ontario Drinking Water Quality Standards (ODWQS). Certificates of lab analysis are provided as an attachment. Parameters reported at concentrations exceeding ODWQS criteria are outlined in Table 3.

Table 3 Summary of Water Quality Parameters

Parameter	ODWQS Criteria	TW3
Hardness	100 mg/L	**347 mg/L**

1. *ODWQS Criteria are for Aesthetic/Operational objectives*

Notably there were no E.Coli. or total coliform bacteriological detections in the water sample. Hardness exceeded the aesthetic/operational objectives outlined in the ODWQS. Hardness can be treated through the use of conventional water softening units.

DISCUSSION OF RESULTS

It is concluded that test well TW3 is suitable for water supply for the proposed development.

The well is able to provide water at a rate of 26.5 L/min, which equates to a volume of 9,540 L/day. Procedure D-5-5 requires a rate of 13.7 L/min with a daily volume of 2,750 for a 4-bedroom dwelling with an ARU. Additional drawdown was available at the end of the test, and recovery of 95% was recorded within an hour of the end of the test.

No significant influences from water withdrawal at TW3 were noted at adjacent wells. The water level responses that were noted are considered negligible as there is ample wellbore storage in the wells. As such, there are considered to be sufficient groundwater resources available at the Site to support the proposed development.

The water quality was considered to be relatively good. The only parameter to exceed ODWQS guidelines was hardness, which can typically be treated with commonly available residential grade water treatment equipment.

March 11, 2026

CLOSING

We trust that the information contained in this letter meets your current requirements. If you have questions or comments regarding, please do not hesitate to contact the undersigned at (705) 742-7900.

Best regards,

Cambium Inc.

DocuSigned by:
7442FDF6DA3049B...

Holly Warren, B.Sc. (Hons.),
Junior Hydrogeologist

DocuSigned by:
6C8CA15FD6B4444...

Warren Young, P.Eng.,
Coordinator – Hydrogeologist

DocuSigned by:
677F3F2E4427404...

Kevin Warner, M.Sc., P.Geo (Ltd), BCIN
Technical Lead - Hydrogeology

HW/WY/KW

Encl. *Cambium Qualifications and Limitations*
Laboratory Certificate of Analysis for TW3
TW3 Well Record
Figure 1 Site Plan
Figure 2 TW3 Pump Test – November 13, 2025

Copies: *Scott Jeffery*
Ray Jeffery
Debbie Vandenakker
Nicole Mountain
Michael Carswell

P:\17900 to 17999\17986-003 Jeffery Homes - WWW - Pt. Lot 19, Conc. 19\Deliverables\REPORT - Addendum Pumping Test\FINAL\2026-03-11 LTR Addendum - TW3 Pumping Test.docx

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March 11, 2026

REFERENCES

Cambium. (2026). *Technical Response to Public Comments for Part of Lot 19, Concession 19, Municipality of Trent Lakes, County of Peterborough.* Cambium Inc.

Jp2g Consultants. (2021). *Hydrogeological Investigation and Terrain Analysis: Proposed Anderson Subdivision.* Jp2g Consultants.

MECP. (1996). *Technical Guideline For Private Wells: Water Supply Assessment.*

March 11, 2026

CAMBIUM QUALIFICATIONS AND LIMITATIONS

Limited Warranty

In performing work on behalf of a client, Cambium relies on its client to provide instructions on the scope of its retainer, and, on that basis, Cambium determines the precise nature of the work to be performed. Cambium undertakes all work in accordance with applicable accepted industry practices and standards. Unless required under local laws, other than as expressly stated herein, no other warranties or conditions, either expressed or implied, are made regarding the services, work or reports provided.

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Facts, conditions, information, and circumstances may vary with time and locations and Cambium's work is based on a review of such matters as they existed at the particular time and location indicated in its reports. No assurance is made by Cambium that the facts, conditions, information, circumstances, or any underlying assumptions made by Cambium in connection with the work performed will not change after the work is completed and a report is submitted. If any such changes occur or additional information is obtained, Cambium should be advised and requested to consider if the changes or additional information affect its findings or results.

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Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

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Potential liability to the client arising out of the report is limited to the amount of Cambium's professional liability insurance coverage. Cambium shall only be liable for direct damages to the extent caused by Cambium's negligence and/or breach of contract. Cambium shall not be liable for consequential damages.

Personal Liability

The client expressly agrees that Cambium employees shall have no personal liability to the client with respect to a claim, whether in contract, tort and/or other cause of action in law. Furthermore, the client agrees that it will bring no proceedings nor take any action in any court of law against Cambium employees in their personal capacity.

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FINAL REPORT

CA14050-NOV25 R1

17986-003, Bobc.aygeon

Prepared for

Cambium Inc.

FINAL REPORT

CA14050-NOV25 R1

First Page

CLIENT DETAILS		LABORATORY DETAILS	
Client	Cambium Inc.	Project Specialist	Brad Moore Hon. B.Sc
Address	194 Sofia Street Peterbrough, ON K9H 1E3, Canada	Laboratory	SGS Canada Inc.
		Address	185 Concession St., Lakefield ON, K0L 2H0
Contact	Warren Young		
Telephone	705-742-7900	Telephone	705-652-2143
Facsimile	705-742-7907	Facsimile	705-652-6365
Email	warren.young@cambium-inc.com; file@cambium-inc.com; ESc	Email	brad.moore@sgs.com
Works #		SGS Reference	CA14050-NOV25
Project Reference	17986-003, Bobc.aygeon	Received	2025-11-13
		Approved	11/19/2025
Batch		Report Number	CA14050-NOV25 R1
Samples	WATER (2)	Date Reported	11/19/2025

COMMENTS

Temperature of Sample upon Receipt: 8 degrees C
Cooling Agent Present:Yes
Custody Seal Present:Yes

Chain of Custody Number:044795

SIGNATORIES

Brad Moore Hon. B.Sc

FINAL REPORT

TABLE OF CONTENTS

FINAL REPORT

CA14050-NOV25 R1

Client: Cambium Inc.
Project: 17986-003, Bobc.aygeon
Project Manager: Warren Young
Samplers: Jenacy Samways

MATRIX: WATER

L1 = ODWS_AO_OG / WATER / - - Table 4 - Drinking Water - Reg O.169_03

L2 = ODWS_MAC / WATER / - - Table 1,2 and 3 - Drinking Water - Reg O.169_03

Sample Number	9	10
Sample Name	TW3	TW3_DUP
Sample Matrix	Ground Water	Ground Water
Sample Date	2025-11-13 00:00	2025-11-13 00:00

Parameter	Units	RL	L1	L2	Result	Result
General Chemistry						
Alkalinity	mg/L as CaCO3	2	500		300	---
Carbonate	mg/L as CaCO3	2			< 2	---
Bicarbonate	mg/L as CaCO3	2			300	---
Colour	TCU	3	5		< 3	---
Conductivity	uS/cm	2			702	---
Total Dissolved Solids	mg/L	30	500		397	---
Turbidity	NTU	0.10	5	1	0.15	---
Dissolved Organic Carbon	mg/L	1	5		1	---
Ammonia+Ammonium (N)	as N mg/L	0.1			< 0.1	---
Metals and Inorganics						
Sulphate	mg/L	2	500		45	---
Nitrite (as N)	as N mg/L	0.03		1	< 0.03	---
Nitrate (as N)	as N mg/L	0.06		10	< 0.06	---
Nitrate + Nitrite (as N)	as N mg/L	0.06			< 0.06	---
Hardness	mg/L as CaCO3	0.05	100		347	---
Calcium (total)	mg/L	0.01			128	---
Iron (total)	mg/L	0.007	0.3		0.012	---
Magnesium (total)	mg/L	0.001			6.90	---
Manganese (total)	mg/L	0.00001	0.05		0.0108	---
Sodium (total)	mg/L	0.01	200	20	16.2	---

FINAL REPORT

CA14050-NOV25 R1

Client: Cambium Inc.
Project: 17986-003, Bobc.aygeon
Project Manager: Warren Young
Samplers: Jenacy Samways

MATRIX: WATER

L1 = ODWS_AO_OG / WATER / - - Table 4 - Drinking Water - Reg O.169_03

L2 = ODWS_MAC / WATER / - - Table 1,2 and 3 - Drinking Water - Reg O.169_03

Sample Number					9	10
Sample Name					TW3	TW3_DUP
Sample Matrix					Ground Water	Ground Water
Sample Date					2025-11-13 00:00	2025-11-13 00:00
Parameter	**Units**	**RL**	**L1**	**L2**	**Result**	**Result**
Microbiology						
Ecoli	mpn/100mL	0		0	0	0
Total Coliform	MPN/100mL	0		0	0	0
Fecal Coliform	mpn/100mL	0			0	---
Other (ORP)						
pH	No unit	0.05	8.5		7.72	---
Chloride	mg/L	1	250		31	---

FINAL REPORT

EXCEEDANCE SUMMARY

Parameter	Method	Units	Result	ODWS_AO_OG / WATER / - - Table 4 - Drinking Water - Reg O.169_03 L1	ODWS_MAC / WATER / - - Table 1,2 and 3 - Drinking Water - Reg O.169_03 L2
TW3					
Hardness	SM 3030/EPA 200.8	mg/L as CaCO3	347	100	

FINAL REPORT

QC SUMMARY

Alkalinity

Method: SM 2320 I Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Alkalinity	EWL0339-NOV25	mg/L as CaCO3	2	< 2	0	20	104	80	120	NA		

Ammonia by SFA

Method: SM 4500 I Internal ref.: ME-CA-IENVISFA-LAK-AN-007

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Ammonia+Ammonium (N)	SKA0156-NOV25	as N mg/L	0.1	<0.1	ND	10	101	90	110	105	75	125

FINAL REPORT

QC SUMMARY

Anions by discrete analyzer

Method: US EPA 325.2 | Internal ref.: ME-CA-[ENV]EWL-LAK-AN-026

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Chloride	DIO8019-NOV25	mg/L	1	<1	0	20	98	70	130	94	70	130
Sulphate	DIO8019-NOV25	mg/L	2	<2	1	20	107	80	120	105	75	125

Anions by IC

Method: EPA300/MA300-Ions1.3 | Internal ref.: ME-CA-[ENV]IC-LAK-AN-001

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Nitrate + Nitrite (as N)	DIO0381-NOV25	mg/L	0.06	<0.06	NA		NA			NA		
Nitrite (as N)	DIO0381-NOV25	mg/L	0.03	<0.03	ND	20	104	80	120	101	70	130
Nitrate (as N)	DIO0381-NOV25	mg/L	0.06	<0.06	ND	20	103	80	120	100	70	130

FINAL REPORT

QC SUMMARY

Carbon by SFA

Method: SM 5310 | Internal ref.: ME-CA-[ENV]SFA-LAK-AN-009

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Dissolved Organic Carbon	SKA0164-NOV25	mg/L	1	<1	1	20	96	90	110	97	75	125

Carbonate/Bicarbonate

Method: SM 2320 | Internal ref.: ME-CA-[ENV]EWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Carbonate	EWL0339-NOV25	mg/L as CaCO3	2	< 2	ND	10	NA	90	110	NA		
Bicarbonate	EWL0339-NOV25	mg/L as CaCO3	2	< 2	0	10	NA	90	110	NA		

FINAL REPORT

QC SUMMARY

Colour

Method: SM 2120 | Internal ref.: ME-CA-[ENV]EWL-LAK-AN-002

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Colour	EWL0346-NOV25	TCU	3	< 3	ND	10	105	80	120	NA		

Conductivity

Method: SM 2510 | Internal ref.: ME-CA-[ENV]EWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Conductivity	EWL0339-NOV25	uS/cm	2	2	0	10	99	90	110	NA		

FINAL REPORT

QC SUMMARY

Metals in aqueous samples - ICP-MS

Method: SM 3030/EPA 200.8 | Internal ref.: ME-CA-IENVISPE-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Calcium (total)	EMS0185-NOV25	mg/L	0.01	<0.01	1	20	94	90	110	80	70	130
Iron (total)	EMS0185-NOV25	mg/L	0.007	<0.007	1	20	97	90	110	100	70	130
Magnesium (total)	EMS0185-NOV25	mg/L	0.001	<0.001	0	20	97	90	110	96	70	130
Manganese (total)	EMS0185-NOV25	mg/L	0.00001	<0.00001	1	20	99	90	110	73	70	130
Sodium (total)	EMS0185-NOV25	mg/L	0.01	<0.01	1	20	97	90	110	94	70	130

Microbiology

Method: SM 9223B | Internal ref.: ME-CA-IENVIMIC-LAK-AN-021

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%)		Spike Recovery (%)	Recovery Limits (%)	
								Low	High		Low	High
Ecoli	BAC9214-NOV25	mpn/100mL	-	ACCEPTED	ACCEPTED							
Fecal Coliform	BAC9214-NOV25	mpn/100mL	-	ACCEPTED	ACCEPTED							
Total Coliform	BAC9214-NOV25	MPN/100mL	-	ACCEPTED	ACCEPTED							

FINAL REPORT

QC SUMMARY

pH

Method: SM 4500 I Internal ref.: ME-CA-IENVIEWL-LAK-AN-006

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High
pH	EWL0339-NOV25	No unit	0.05	NA	0		100			NA		

Solids Analysis

Method: SM 2540C I Internal ref.: ME-CA-IENVIEWL-LAK-AN-005

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High
Total Dissolved Solids	EWL0363-NOV25	mg/L	30	<30	5	20	96	80	120	NA		

Turbidity

Method: SM 2130 I Internal ref.: ME-CA-IENVIEWL-LAK-AN-003

Parameter	QC batch Reference	Units	RL	Method Blank	Duplicate		LCS/Spike Blank			Matrix Spike / Ref.		
					RPD	AC (%)	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High	Spike Recovery (%)	Recovery Limits (%) Low	Recovery Limits (%) High
Turbidity	EWL0329-NOV25	NTU	0.10	< 0.10	0	10	99	90	110	NA		

FINAL REPORT

QC SUMMARY

Method Blank: a blank matrix that is carried through the entire analytical procedure. Used to assess laboratory contamination.

Duplicate: Paired analysis of a separate portion of the same sample that is carried through the entire analytical procedure. Used to evaluate measurement precision.

LCS/Spike Blank: Laboratory control sample or spike blank refer to a blank matrix to which a known amount of analyte has been added. Used to evaluate analyte recovery and laboratory accuracy without sample matrix effects.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate laboratory accuracy with sample matrix effects.

Reference Material: a material or substance matrix matched to the samples that contains a known amount of the analyte of interest. A reference material may be used in place of a matrix spike.

RL: Reporting limit

RPD: Relative percent difference

AC: Acceptance criteria

Multielement Scan Qualifier: as the number of analytes in a scan increases, so does the chance of a limit exceedance by random chance as opposed to a real method problem. Thus, in multielement scans, for the LCS and matrix spike, up to 10% of the analytes may exceed the quoted limits by up to 10% absolute and the spike is considered acceptable.

Duplicate Qualifier: for duplicates as the measured result approaches the RL, the uncertainty associated with the value increases dramatically, thus duplicate acceptance limits apply only where the average of the two duplicates is greater than five times the RL.

Matrix Spike Qualifier: for matrix spikes, as the concentration of the native analyte increases, the uncertainty of the matrix spike recovery increases. Thus, the matrix spike acceptance limits apply only when the concentration of the matrix spike is greater than or equal to the concentration of the native analyte.

FINAL REPORT

LEGEND

FOOTNOTES

NSS	Insufficient sample for analysis.
RL	Reporting Limit.
↑	Reporting limit raised.
↓	Reporting limit lowered.
NA	The sample was not analysed for this analyte
ND	Non Detect

Results relate only to the sample tested.

Data reported represent the sample as submitted to SGS.

Test method information available upon request.

"Temperature Upon Receipt" is representative of the whole shipment and may not reflect the temperature of individual samples.

SGS Canada Inc. statement of conformity decision rule does not consider uncertainty when analytical results are compared to a specified standard or regulation.

-- End of Analytical Report --

...st for Laboratory Services and CHAIN OF CUSTODY

SGS Industries & Environment - Lakefield: 185 Concession St, Lakefield, ON K0L 2H0 Phone: 705-652-2000 Fax: 705-652-6365 Web: www.sgs.com/environment
- London: 657 Consortium Court, London, ON, N6E 2S8 Phone: 519-672-4500 Toll Free: 877-848-8060 Fax: 519-672-0361

No: 044795

Page 1 of 1

Laboratory Information Section - Lab use only

Received By: _____
Received Date: NOV 13 2025 (mm/dd/yy)
Received Time: 17 : 30 (hr : min)
Received By (signature): _____
Custody Seal Present: Yes ☑ No ☐
Custody Seal Intact: Yes ☑ No ☐
Cooling Agent Present: Yes ☑ No ☐ Type: ICE Bag
Temperature Upon Receipt (°C) 6.8 9.
CA 14050-NOV25
LAB LIMS #: ~~6.8 9~~

REPORT INFORMATION	INVOICE INFORMATION
Company: CAMBIUM INC	☑ (same as Report Information)
Contact: WARREN YOUNG, JENACY SAMWAYS	Company:
Address: 194 SOPHIA ST, PTBO	Contact:
Phone:	Address:
Fax: warren.young@cambium-inc.com	Phone:
Email: jenacy.samways@cambium-inc.com	Email:

Quotation #: 2025 6086 P.O. #:
Project #: 17986-003 Site Location/ID: BOBCAYGEON

TURNAROUND TIME (TAT) REQUIRED

☐ Client Regular TAT ☑ Regular TAT (5-7days)

TAT's are quoted in business days (exclude statutory holidays & weekends). Samples received after 6pm or on weekends: TAT begins next business day

RUSH TAT (Additional Charges May Apply): ☐ 1 Day ☐ 2 Days ☐ 3 Days ☐ 4 Days

PLEASE CONFIRM RUSH FEASIBILITY WITH SGS REPRESENTATIVE PRIOR TO SUBMISSION

Specify Due Date: _____

***NOTE:** DRINKING (POTABLE) WATER SAMPLES FOR HUMAN CONSUMPTION MUST BE SUBMITTED WITH SGS DRINKING WATER CHAIN OF CUSTODY

REGULATIONS

☐ O.Reg 153/04 ☐ O.Reg 406/19
☐ Table 1 ☐ Res/Park Soil Texture: ☐ Coarse
☐ Table 2 ☐ Ind/Com ☐ Medium/Fine
☐ Table 3 ☐ Agri/Other
☐ Table _____ Appx. _____
Soil Volume ☐ <350m3 ☐ >350m3

Other Regulations:
☐ Reg 347/558 (3 Day min TAT)
☐ PWQO ☐ MMER
☐ CCME ☐ Other: _____
☐ MISA
☑ ODWS Not Reportable *See note

Sewer By-Law:
☐ Sanitary
☐ Storm
Municipality: _____

RECORD OF SITE CONDITION (RSC) ☐ YES ☐ NO

ANALYSIS REQUESTED

	SAMPLE IDENTIFICATION	DATE SAMPLED	TIME SAMPLED	# OF BOTTLES	MATRIX	Field Filtered (Y/N)	M&I: Metals & Inorganics incl CrVI, CN,Hg pH,(B(HWS),EC,SAR–soil) (Cl, Na–water)	M&I: Full Metals Suite ICP metals plus B(HWS-soil only) Hg, CrVI	M&I: ICP Metals only Sb,As,Ba,Be,B,Cd, Cr,Co,Cu,Pb,Mo,Ni,Se,Ag,Tl,U,V,Zn	SVOC: PAHs only	SVOC: SVOCs all incl PAHs, ABNs, CPs	PCB: PCBs Total ☐ Aroclor ☐	PHC: F1-F4 + BTEX	PHC: F1-F4 only no BTEX	VOC: VOCs all incl BTEX	VOC: BTEX only	Pest: Pesticides Organochlorine or specify other	Other: AS PER QUOTE #	Other: ECLTC	Other	Other	Other: Sewer Use: Specify pkg:	Other: Water Characterization Pkg General ☐ Extended ☐	SPLP: Specify tests ☐Metals ☐VOC ☐1,4-Dioxane ☐OCP ☐ABN	TCLP: Specify tests ☐M&I ☐VOC ☐PCB ☐B(a)P ☐ABN ☐Ignit.	COMMENTS:
1	TW3	11/13/2025	15:20	13	GW	N												✓							
2	TW3_Dup	11/13/2025	15:20	1	GW	N													✓							
3																									
4																									
5																									
6																									
7																									
8																									
9																									
10																									
11																									
12																									

Observations/Comments/Special Instructions

(*) Please use OCFU/100mL detection limit for bacteria samples (*)*

Sampled By (NAME): Jenacy Samways Signature: Jenacy Samways Date: 11 / 13 / 25 (mm/dd/yy)

Relinquished by (NAME): Jenacy Samways Signature: Jenacy Samways Date: 11 / 13 / 25 (mm/dd/yy)

Pink Copy - Client
Yellow & White Copy - SGS

Revision #: 1.8
Date of Issue: 06 SEP 2024

Ontario — Ministry of the Environment, Conservation and Parks

Well Tag No. (Place Sticker and/or Print Below) A415246 Tag#:A415246

Well Record

Regulation 903 Ontario Water Resources Act

Measurements recorded in: ☐ Metric ☒ Imperial

Well Owner's Information

First Name	Last Name/Organization	E-mail Address	☐ Well Constructed by Well Owner
	Jeffery Homes		

Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
1200 Airport Blvd, Suite 201	Oshawa	ON		

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
168 County Rd. 49	Trent Lakes (Harvey)	19	19

County/District/Municipality	City/Town/Village	Province	Postal Code
Peterborough	Bobcaygeon	Ontario	K0M 1A0

UTM Coordinates	Zone	Easting	Northing	Municipal Plan and Sublot Number	Other
NAD 8 3	17	6948738	4937391		

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	Depth (m/ft) To
				0	240
	See Attached				

Annular Space

Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m^3/ft^3)
0	3	Bentonite Chips	0.71
3	20	Bentonite Grout	15

Method of Construction

☐ Cable Tool ☐ Diamond
☐ Rotary (Conventional) ☐ Jetting
☐ Rotary (Reverse) ☐ Driving
☐ Boring ☐ Digging
☒ Air percussion
☒ Other, specify Dual rotary

Well Use

☐ Public ☐ Commercial ☐ Not used
☒ Domestic ☐ Municipal ☐ Dewatering
☐ Livestock ☐ Test Hole ☐ Monitoring
☐ Irrigation ☐ Cooling & Air Conditioning
☐ Industrial
☐ Other, specify

Construction Record - Casing

Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	Depth (m/ft) To
6.25	steel	0.188	+3	25.5
6	open hole		25.5	240

Construction Record - Screen

Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	Depth (m/ft) To

Status of Well

☒ Water Supply
☐ Replacement Well
☐ Test Hole
☐ Recharge Well
☐ Dewatering Well
☐ Observation and/or Monitoring Hole
☐ Alteration (Construction)
☐ Abandoned, Insufficient Supply
☐ Abandoned, Poor Water Quality
☐ Abandoned, other, specify
☐ Other, specify

Water Details

Water found at Depth	Kind of Water	Hole Diameter Depth From	Depth To	Diameter (cm/in)
111 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify	0	20	10
192–198 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify	20	240	6
223–224 (m/ft) ☐ Gas	☒ Fresh ☒ Untested ☐ Other, specify			

Results of Well Yield Testing

After test of well yield, water was:
☒ Clear and sand free
☐ Other, specify

If pumping discontinued, give reason:

Pump intake set at (m/ft): 230

Pumping rate (l/min / GPM): 7

Duration of pumping: 6 hrs + min

Final water level end of pumping (m/ft): 64.54

If flowing give rate (l/min/GPM):

Recommended pump depth (m/ft): 230

Recommended pump rate (l/min/GPM): 7

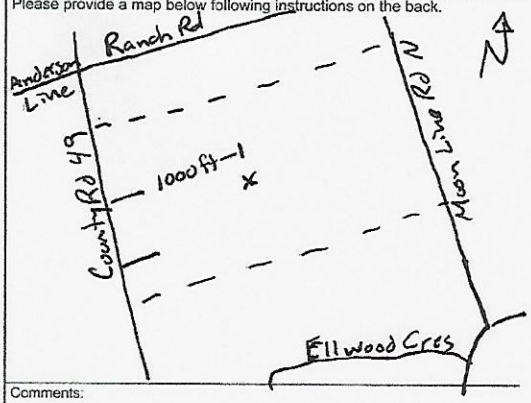
Well production (l/min/GPM): 7

Disinfected? ☒ Yes ☐ No

Draw Down Time (min)	Draw Down Water Level (m/ft)	Recovery Time (min)	Recovery Water Level (m/ft)
Static Level	45.20		
1	49.15	1	59.00
2	51.27	2	55.55
3	52.86	3	52.90
4	54.15	4	50.98
5	55.18	5	49.66
10	58.40	10	47.40
15	60.07	15	46.94
20	61.10	20	46.67
25	61.68	25	46.50
30	62.12	30	46.38
40	62.78	40	46.24
50	63.10	50	46.17
60	63.34	60	46.15

Map of Well Location

Please provide a map below following instructions on the back.



Comments:

Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
G. Hart & Sons Well Drilling Ltd.	2662

Business Address (Street Number/Name)	Municipality	Province	Postal Code	Business E-mail Address
PO Box 850 Fenelon Falls		Ontario	K0M 1N0	ghart ghart.ca

Bus. Telephone No. (inc. area code)	Name of Well Technician (Last Name, First Name)	Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
705 887 3331	Robinson, Brandon / Watson, Bryan	4802 / 2441		

Well owner's information package delivered	Date Package Delivered	Date Work Completed
☒ Yes ☐ No	mailed	2025 11 13

Ministry Use Only

Audit No. Z437433

Received

Ministry's Copy

G. HART & Son
Well Drilling Ltd.
Box 850, Fenelon Falls, Ont.
K0M 1N0

Tel:(705)-887-3331
Fax:(705)887-4788
E-Mail ghart@ghart.ca

WELL LOG

WELL TAG # :A415246

168 County Road 49, Bobcaygon,ON

General Colour	Most Common Material	Other Materials	General Description	Depth From (Feet)	Depth To (Feet)
Black	Topsoil			0	4
Brown	Silty sand & Gravel			4	21
Grey	Clay & Gravel			21	23
Grey	Broken Limestone			23	24
Grey	Limestone			24	137
White & Black	Limestone			137	164
Grey & Green	Limestone			164	181
Grey & White	Limestone			181	196
Grey & Red	Limestone			196	198
Green	Limestone			198	199
Green & Red	Limestone			199	203.5
Grey	Limestone			203.5	204
Green & Red	Limestone			204	204.5
Red	Limestone			204.5	205
Grey, Red & Green	Limestone			205	206
Brown & Red	Limestone			206	207.5
Red	Limestone			207.5	214
Grey, Pink & Black	Granite			214	240

TECHNICAL RESPONSE TO PUBLIC COMMENTS

JEFFREY HOMES
Pt Lot 19, Conc. 19 – 168 County Road 49
Bobcaygeon, Ontario

LEGEND

- Monitoring Well
- Test Well
- Pumping Test Discharge Direction
- Wetland Boundary (D.G. Biddle & Associates)
- Site (approximate)

Notes:
- This document contains information licensed under the Open Government License - Ontario.
- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.
- Cambium Inc. makes every effort to ensure this map is free from errors but cannot be held responsible for any damages due to error or omissions. This map should not be used for navigation or legal purposes. It is intended for general reference use only.

CAMBIUM
194 Sophia Street
Peterborough, Ontario, K9H 1E5
Tel: (705) 742.7900 Fax: (705) 742.7907
www.cambium-inc.com

SITE PLAN

Project No.:	Date:
24373-001	November 2025
Scale: 1:6,000	Projection: NAD 1983 UTM Zone 17N

Created by:	Checked by:	Figure:
DBC	WY	1

O:\GIS\MXDs\17900-17999\17986-003 Jeffery Homes - WWW - Pt. Lot 19 Conc. 19\2025-11-26 Technical Response.aprx

Figure 2: TW3 Pump Test - November 13, 2025